INFLUENCE OF TREAD DEPTH ON WET SKID RESISTANCE OF TIRES

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Bald tires have lower skid resistance compared to tires with full tread depth. As could be expected tires with very low tread depths are reported to be more frequently involved in wet weather accidents. An overview of the legal requirements for the minimum value of the tread depth in several countries is given for car as well as truck and bus tires.

The influence of tread depth is not linear and depends strongly on vehicle speed and road surface characteristics. Measurements carried out in the U.K., Germany and the Netherlands are described and the different measuring methods are discussed. The measurements for car tires deal with the peak value of the braking force, the locked wheel value, as well as the side force coefficient.

The influence of tread depth of truck and bus tires has been investigated in the U.K. and the Netherlands as far as the braking force coefficients are concerned.

Especially in the lower tread depth range, the influence of tread depth on skid resistance is very important, for both car and truck tires. The results lead to the conclusion that a legislative minimum permissible tread depth can contribute to less skidding accidents. Although the level of this minimum value is theoretically somewhat arbitrary, practical considerations suggest that a minimum tread depth of 1,6 mm (2/32 inch) seems to be a sound limit.

Skid Resistance Coefficients

It is a well-known fact that the wet skid resistance of smooth tires is poor compared with tires with full tread depth (e.g. 1,2). The decrease in skid resistance however is not linearly dependant on the tread depth. Several investigations into the variation of wet skid resistance with tread depth have been carried out and will be discussed in this paper.

Wet skid resistance can be characterized by three coefficients, two braking force coefficients μ xm and μ xb and a side force coefficient μ y. The braking force coefficient and the side force coefficient are defined respectively as the instantaneous values of braking force and the side force, divided by the vertical load operating at that instant.

The three coefficients are:

μ _{xm} = .	(Fx/Fz) _{max} ,	the maximum value of the braking force coefficient before locking.
μ _{xb} =	(Fx/Fz) ,	the average braking force coefficient when the wheel is locked.
μ _y =	(Fy/Fz) ,	the average side force coefficient at a slip angle exceeding, say, 8 deg.

Under different circumstances each of these coefficients is of overriding importance. A high value for μ xm means that the brakes can be applied with force without the wheels locking. This situation is frequently encountered in nose-to-tail driving when it is often necessary to brake hard without there being any question of an emergency. A high U xm value is also of importance in connection with anti-locking devices. If, on the other hand, an emergency arises, the driver will usually brake as hard as he can, causing the wheels to lock and thereby endanger the stability of the vehicle. In this situation a high \coprod_{xb} value is required so as to achieve a short braking distance. On wet road surfaces µxb is always considerably lower than μ xm. A high μ y value is desirable when a driver wishes or needs to change direction and furthermore to promote stability during braking before the wheel locks.

Legislation

In many countries a minimum permissible tread depth has been fixed by law (table 1.)

	car tires	truck and bus tires			
Austria	1.6	2			
Belgium	1	1			
Denmark	1 ^a	1 ^a			
Finland	1	1			
France	1 ^a	1 ^a			
Germany	1 ^a	1			
Ireland	1	1			
Italy	1	1			
Japan	1.6	1.6			
Netherlands	visible tread	no visible			
	pattern	carcass			
Norway	1	1			
Portugal	1	1			
Spain	1	1			
Sweden	1	1			
Switzerland	1_	1			
United Kindom	1 ^a	1 ^a			
USA	1.6	1.6			
USSR	1	1			

a. This minimum tread depth must be present throughout 3/4 of the width of the tread.

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The European Tire and Rim Technical Organization (ETRTO) has recommended that the minimum tread depth of 1 mm should cover at least 3/4 of the width of the tread. The Economic Commission for Europe (ECE) of the United Nations recommends a minimum tread depth of 1 mm throughout the full width of the tread. In some countries however there is a tendency to require a higher value of the minimum tread depth.

The arguments put forward are generally more subjective rather than objective ones. A good case for a minimum permissible tread depth should be based upon an accident analysis in this respect. Unfortunately little reliable data are available on the question of involvement of tires with different tread depths at the sites of wet skid accidents.

Accident involvement of low tread depth tires

In Texas wet weather accidents were analyzed and the most important contributing factors determined (3). With respect to tire tread depth it was found that accident vehicles generally had less tread depth compared to the average vehicle in the study area. Rear wheels had generally remarkably lower tread depth compared to front wheels, 40% of the rear wheels had a tread depth less than 3,2 mm compared to about 25% of the front wheels.

Table 2. Percentages of inspected vehicles with low tread depths.

		1 po dent		icle	sam	ple	vehi	cle
	LF	RF	LR	RR	LF	RF	LR	RR
tread depth less than 1.6 mm	11	11	24	24	9	9	20	18
tread depth less than 3.2 mm	29	27	42	42	24	24	38	37

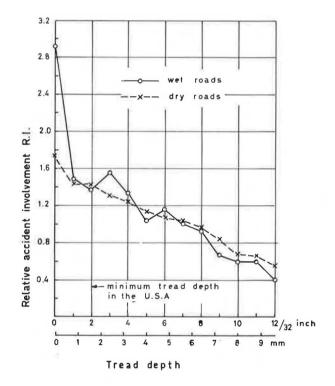
From table 2 it is clear that the percentage of low tread depth tires involved in wet weather accidents is significantly higher than the presence of these tread depths at random inspections. It seems that low tread depth is an important factor contributing to wet weather accidents.

A report of the Highway Safety Foundation in Ohio brings out the involvement of low tread depth tires in car accidents even more strongly (4). A statistical survey was carried out whereby relative accident involvement (R.I.) was determined. This was defined as follows:

R.I. = the percentage of tires with a particular tread depth of cars involved in accidents, divided by

the percentage of tires of this tread depth found during random inspection of vehicles. It was established that on wet road surfaces tires with tread depths of less than 0.8 mm (1/32 inch) were involved in accidents to a far greater extent than tires with greater tread depths (Fig. 1).

Figure 1. Relative accident involvement as a function of tire tread depth.



Also on dry surfaces the relative involvement 'in accidents of tires increase with decreasing tread depth, even though both the braking force coefficient and the side force coefficient usually increase under these circumstances. This implies that factors other than tread depth alone may also play a role in accidents. The sharply rising relative accident involvement on wet roads of tires with lower tread depths is explained by the much reduced braking force coefficients under these circumstances. In this report an attempt has been made to calculate a cost-benefit factor in order to determine a critical value for the minimum tread depth. The method adopted is, however, very debatable. A distinction between trucks and busses and passenger cars was not made in the accident analyses before mentioned. The Ohio report deals only with car accidents.

Influence of tread depth of car tires on wet skid resistance

Only a few papers deal with the influence of tread depth on wet skid resistance. Some investigators have reported measurements of one tire on one specific road surface as a part of a general investigation of wet skid resistance $(\underline{2}, \underline{5})$. From these measurements no general conclusions could be drawn.

After 1970 more specific investigations into the influence of tread depth have been published; these were carried out in the United Kingdom, Germany and the Netherlands.

United Kingdom

In the United Kingdom the Road Research Laboratory has measured a particular type of crossply tire on six different road surfaces (6). The different tread depths were obtained by wearing the treads under normal traffic conditions. The drawback of this procedure is that the entire measurement was spread over an extended period of time, as a result of which measuring was done under widely varying weather conditions. It is true that adjustments were made so as to allow for variations in ambient temperature, by comparing the results with a reference bald tire, but it is our experience that measurements performed under widely differing weather conditions are difficult to interpret. Further limitations were that only one type of tire, a cross-ply tire, was tested, which is less interesting than a radial-ply tire, and that only braking force coefficients were measured.

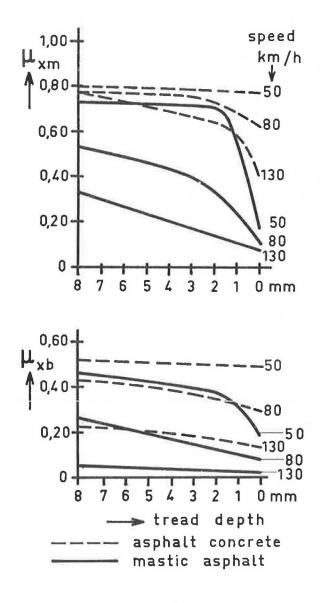
The measurements were carried out by mounting the test tires on the front wheels of a high speed vehicle. The braking system had been modified such that the front brakes could be applied alone at a controlled rate until these wheels locked. The vehicle was allowed to coast immediately prior to the brake application so that the deceleration during coasting could be established. The additional deceleration during braking is a measure of the braking force. In the evaluation of the results the weight transfer to the front axle was taken into account.

A disadvantage of this method as far as the peak coefficient is concerned is that the deceleration of the vehicle is measured and the brake forces are averaged over the two tires. For the locked wheel values this would be true but as the peak braking force only appears for 1/10 of a second or less it is highly unlikely that this would occur exactly at the same time for both tires, on account of the dynamic wheel load variations. Therefore it may be expected that the μ xm values measured in this way are lower than the true values for the individual tires. The measurements were carried out on 6 road surfaces ranging from smooth mastic asphalt to a rough and sharp aggregate, with a 5.60 - 13 tire, load 3500 N, inflation pressure 180 kN/m². The water depth on all surfaces was about about 0.5 mm except on the mastic asphalt where a water layer with a depth of 1.1 mm was found.

This was due to the road surface texture and drainage. The water depth was defined as the average depth of water standing above the aggregate of each of the test surfaces. The surfaces were maintained in a wetted condition using a spray bar facility. All the surfaces were part of a special test track.

In figure 2 some of the results are presented. Under certain conditions there appeared to be a very sharp drop in skid resistance when the tire wears to a tread depth below 1 or 2 mm. This effect is very pronounced in the peak values μ xm.

Figure 2. Skid resistance as a function of tire tread depth. (Measurements in the U.K.)



Germany

In Germany the influence of tire tread depth on the side force coefficient μ y has been investigated (7). Two types of tires, a cross-ply and a radial ply tire have been tested.

The different tread depths were achieved by grinding down to the desired depth and subsequently running in the tires on public roads.

A normal car chassis was changed in such a way that during driving the front wheels could be set to slip angles in opposite directions in the range from -18 to +18 degrees. The side force was measured with the aid of a dynamometer mounted on the wheel. On a test track 3 surfaces were chosen from smooth concrete to a rather rough surface. On a racing track a very rough surface was available for the measurements. A water spray installation took care to maintain the calculated water depth of 1.5 mm, which may be considered as an extremely thick water layer. The tire sizes were 5.60-15 and 155SR15 with a vertical load of 2350 N and an inflation pressure of 150 kN/m^2 .

Fig. 3 shows a typical set of test results. It can be seen that with decreasing tread depth the side force coefficient gradually decreases. There appears to be no particular point beyond which the μ y value drops progressively. The radial-ply tire shows a remarkably higher μ y value as compared to the cross-ply one.

The Netherlands

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Measurements of braking force coefficients as well as cornering force coefficients have been carried out by the Vehicle Research Laboratory of the Delft University (VRL). The VRL has a test trailer specially built for measuring tire traction characteristics (figure 4). Figure 3. Side force coefficients as a function of tire tread depth. (Measurements in W-Germany)

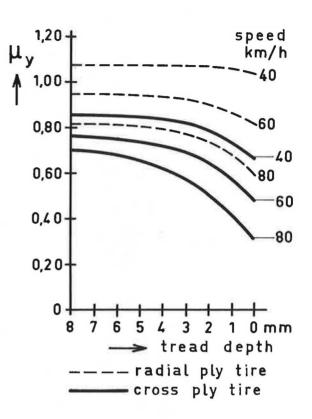
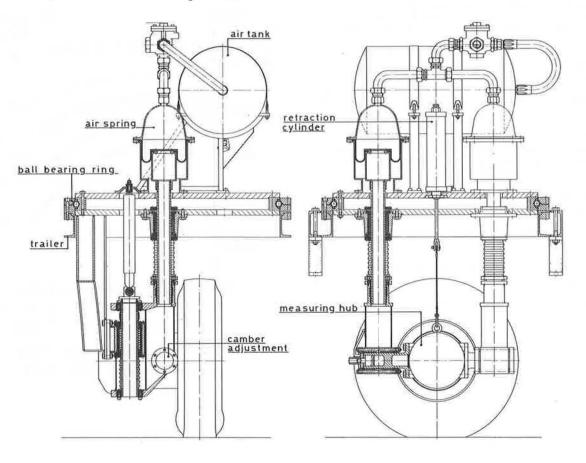


Figure 4. The VRL measuring trailer.



This trailer can steer, camber and brake the test tire. The tire to be measured is mounted on a five component measuring hub, placed in a frame (figure 5).

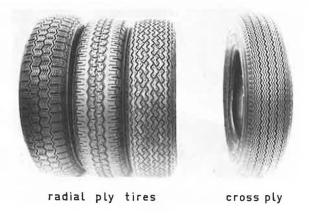
Figure 5. Suspension of the measuring wheel.



A similar frame is equipped with a dummy tire. Both frames can turn in opposite directions to provide slip angles to both tires. Resulting side forces on the trailer are in this way restricted to very low levels. The towing truck has a water tank and by means of air pressure water can be sprayed in front of the test tire. A special nozzle will give a thin flat water layer on the road. A disadvantage of this self supporting watering system is that the water is not at rest with respect to the road surface. But for measurements on public roads it is very difficult to install a water spray installation along the road sections to be measured.

The longitudinal force $F_{\rm X}$ and the side force $F_{\rm y}$ are continuously divided by the prevailing value of the vertical load $F_{\rm Z}$. An electronic device accomplishes this computation and the peak value of the braking force coefficient as well as the average value over the time span that the wheel is locked, are printed out. Also the side force coefficient is averaged over the time that the slip angle exceeds for instance 8 degrees.

Four types of car tires were examined: two textile radial tires and one steel cord radial, all of size 165 SR 13, and one cross-ply tire, size 5.90-13. Figure 6 shows the tread patterns of these tires. Five tires of each type were ground down to the desired tread depth. Figure 6. Car tire tread patterns used for the measurements in the Netherlands.



After grinding, the tires were run in for at least 500 kms. The tires were tested on two types of public road surfaces, A and B (figure 7), at a vertical load of 3300 N and inflation pressure of 180 kN/m².

1000, 1000 1000, 100 friction device that does not measure a friction boundary condition - that is, the skiddometer measures peak braking (constant 0.13 braking slip); the General Motors (GM) trailer, either μ_{max} or μ_{skid} from a pulse braking technique; the Miles trailer, μ_{skid} from a pulse braking technique; and the DBV, µskid from a continuous locked-wheel braking technique. The Mu-Meter, on the other hand, measures cornering force developed on a tire at 7.5° yaw angle. At high pavement friction values, it cannot measure the peak friction boundary condition, whereas for low friction conditions, it may measure cornering force after the peak cornering-force value has been obtained, as shown in figure 26. The data in figure 26 were obtained from reference 31 (p. 654). These data suggest that if the yaw angle for maximum cornering force (limiting coefficient of friction) is exceeded, the cornering force (and cornering friction coefficient) is reduced as yaw angle is further increased. For the case of the Mu-Meter which measures cornering force at 7.5° yaw angle, this type of tire behavior may result in an overestimation of the slipperiness of the wet pavement defined by peak boundary friction conditions.

Aircraft/Ground-Vehicle Correlation

As with ground-vehicle/ground-vehicle correlation attempts, most aircraft/ground-vehicle correlation attempts try to relate the measured output of a friction device with some measured output of the aircraft from data obtained during joint testing of the device and aircraft on artificially wet runway surfaces. Typical aircraft/ground-vehicle relationships obtained from such test programs are shown in figures 27 (Mu-Meter, ref. 24) and 28 (DBV, refs. 11 and 25). Each friction device advocate claims good correlation between the device and the aircraft. For example, reference 26 states that the Mu-Meter may predict aircraft stopping performance within 10 to 15 percent if a correlation ranking system classifying runways surfaces into different texture groups is used. On the other hand, reference 11 states that the DBV can predict aircraft stopping performance within ±15 percent by using its prediction method. The tire friction prediction method (described earlier in the paper) offers another approach to show correlation between ground-vehicle and aircraft measurements of runway slipperiness.

Equation 5 may be modified to the form

 $\mu_{eff} = \eta \, \overline{Y}_{R} \, \mu_{dry}$

where

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µeff effective braking friction coefficient
 realized by the aircraft through its
 antiskid braking system

(8)

µdry characteristic maximum aircraft tire friction coefficient on dry pavement

 η antiskid braking system efficiency, $\label{eq:max} \mu_{\text{eff}}/\mu_{\text{max}}$

This method, using the DBV friction measuring device, is illustrated in figures 29 to 31. The correlation shown in the figures resulted from use of the arbitrarily selected antiskid braking system efficiency model depicted in figure 29 which is patterned after the one described in reference 32.

The data trends shown in figures 29 to 31 suggest that a ground-vehicle friction measuring device can be used to predict the effective friction coefficient an aircraft will develop on a wet runway providing the antiskid braking system efficiency of the aircraft is known. The data trends also suggest that each aircraft type has its own characteristic antiskid braking system efficiency which is dependent upon the landing gear, braking, and antiskid system design.

Summary of Correlation Results

The runway slipperiness research conducted since 1968 in the area of ground-vehicle/ground-vehicle and aircraft/ground-vehicle correlations has been reviewed and yields the following observations:

Ground-vehicle devices that test at constant speed do not correlate well with those devices that test over a speed range.

Ground-vehicle devices that test at constant speed can be correlated together as well as those that test over a speed range regardless of the tire operating mode during testing.

The DBV can be used to predict aircraft tire braking and cornering characteristics on wet runways. Other ground-vehicle devices have the potential to predict these tire characteristics as well if their test procedure is changed from a constant speed test to a speed range test similar to the DBV. Ground-vehicle devices that test at constant speed cannot predict aircraft tire braking and cornering friction coefficient on wet runways over the full take-off and landing speed range of aircraft.

Ground-vehicle and aircraft slipperiness measurements can be correlated. However, the precision of correlation is obtained from artificially wet runway test programs. The accuracy of prediction from the correlation may be degraded when runways are wet from natural rain (different water depths). Further, some of the older aircraft braking systems can allow locked-wheel operation during maximum braking operation on wet runways. The locked-wheel condition can result in reverted rubber hydroplaning which destroys the aircraft/ground-vehicle correlation. For these reasons, predictions of aircraft braking performance on wet runways from groundvehicle devices should be employed only to provide guidance information to pilots.

Status of Runway Slipperiness Measurements

Standard USAF runway skid resistant tests. - Since November 1973, the Air Force Civil Engineering Center (AFCEC) has been measuring the skid resistance properties of airfields. Procedures for conducting the standard skid resistance tests are given in reference 33. This test requires that friction measurements be obtained by both the DBV and Mu-Meter when testing an airfield pavement. AFCEC feels that the friction data obtained from these friction measuring devices are complementary, and together they provide an adequate data base to evaluate the skid resistance of an airfield pavement. AFCEC intends to survey the skid resistance of all USAF runways in the United States and overseas on a periodic basis. AFCEC feels strongly that the concept of using an experienced, well-trained crew and standardized testing procedures for pavement skid

resistance evaluations offers many advantages. This concept requires the Air Force to purchase and maintain a minimum quantity of equipment and ensures that the testing is properly accomplished and documented. Results from this Air Force program are reported in references 28 and 34.

FAA Advisory Circular No. 150/5320-12.- FAA Airports Service issued FAA Advisory Circular No. 150/5320-12 on June 30, 1975 (ref. 35). This advisory circular provides guidance on methods that can be used to provide and maintain airport pavement surface friction characteristics. This guidance is intended for use by airport operators, engineering consultants, and maintenance personnel. This advisory circular does not purport to provide a means to predict aircraft stopping distance. For the requirements specified in this circular, FAA Airports Service requires a friction measuring device which

1. Can provide fast, accurate, and reliable friction values of airport pavement surfaces under varying climatic conditions

2. Can provide a continuous graph record of the pavement surface characteristics

3. Has minimal maintenance and recurring costs

4. Has a simple calibration technique

5. Indicates potential for hydroplaning conditions

This circular is worded carefully such that current friction measuring devices, the DBV for example, are not excluded from use in implementing the circular, although it is clear that the British Mu-Meter is the device favored by FAA Airports Service since it is the only device described in the circular. The advisory circular clearly indicates that its needs are met by a device which measures the relative friction of pavement surfaces and that this measurement of friction does not provide a means to predict aircraft stopping distance (determine how slippery the runway surfaces are for aircraft operation).

It is felt that issuance of this advisory circular by the FAA is a noteworthy step forward in providing guidance to install antihydroplaning runway surfaces at airports. However, the providing of relative friction measurements for engineering and maintenance purposes is secondary to the main objective of a friction evaluation which is to determine how slippery the runway surface is for aircraft operation.

Progress and Problems of Antihydroplaning Runway Surface Treatments

Both runway grooving and porous friction course (PFC) antihydroplaning runway surfaces were originated in England, as described in reference 36. Research on runway grooving in the United States started with NASA experiments in 1962 (reported in ref. 2). PFC pavement research in the United States was initiated by USAF (1972) and is reported in references 37 and 38.

Runway Grooving

Since 1956, approximately 160 runways have been grooved world-wide as indicated in tables 2 to 12. Figure 32 shows the development of grooved runways at U.S. civil airports since the first air carrier airport was grooved in 1967. For the past 3 years an average of 24 air carrier airport runways have been grooved each year. At this present rate, the 224 ILS runways 1524 m (5000 ft) or longer in length at U.S. air carrier airports will all be grooved by 1986. At the present time, six different methods are available for grooving runways, namely, diamond saws, abrasive (carborundum) saws, flails, plastic grooving with segmented drum, plastic grooving with wire comb, and plastic grooving with wire broom. The latter three methods can only be used for grooving portland cement concrete when it has been freshly laid and has not hardened or set up. The most popular grooving method is the diamond saw. Approximately 80 percent of the air carrier airport runways that have been grooved since 1967 have used this grooving method. The effectiveness of runway grooving as an antihydroplaning surface treatment is revealed by reviewing the DBV SDR data shown in tables 13 to 17. Tables 13 to 16 were obtained from reference 39. Table 17 shows data obtained from a recently completed FAA DBV trial application-runway friction calibration and pilot information program (ref. 40). Review of these data suggests that the greatest fraction benefit is realized from closedspaced grooves that are cut 1/4 inch deep in the pavement with diamond saws. This result follows the trend reported in reference 27 where a 25 \times 6 \times 6 mm $(1 \times 1/4 \times 1/4$ in.) pattern was found to be superior to all other patterns studied with regard to preserving traction on wet or flooded runways. Plastic grooving treatments are considered to be an improvement over conventional ungrooved concrete surfaces but are inferior to diamond sawed grooves in both traction performance and water drainage (discussed in section "Flooding on Grooved Runways"). The uniformity of plastic grooving is poor compared with diamond sawed grooves as shown by comparing figures 5 and 6 with figure 33. The data presented in figure 34 compare the traction performance of plastic grooving using a wire comb technique (ref. 41) with other antihydroplaning pavement surface treatments. These data confirm the traction trends just discussed.

The major problem encountered with grooved runways is the chevron cutting of aircraft tires during the touchdown phase of aircraft landings on grooved runways. (See fig. 35.) This problem is discussed in detail in reference 39 and has been studied in reference 42. The civil airlines in the United States at the present time do not consider chevron cutting to be a serious operational problem to their jet transport fleet. It should be noted that the aircraft tire industry has been working in close cooperation with aircraft operators on the chevron cutting problem. During the past 5 years, the aircraft tire industry has developed new tread rubber compounds and tread designs that significantly reduce the degree of chevron cutting on aircraft tires experienced on grooved runways. In this regard, American Airlines reports that over the past 4 years, the number of landings per tire change on its jet transport fleet has increased by 50 percent. During this time period, the number of grooved runways at air carrier airports has increased from 37 to 107. The slipperiness of grooved runways is increased when heavy rubber deposits coat touchdown areas, but this problem is easily corrected by rubber removal treatments (discussed later). Some asphaltic concrete runways have suffered collapsed grooves in trafficked areas. This type of problem is usually created by grooving the asphaltic concrete shortly after the runway has been paved and before the asphaltic concrete has cured properly.

Porous Friction Course

The first PFC surface treatment in the United States was at the Dallas Naval Air Station in 1971 as indicated in table 18. The growth of the PFC surface treatment at U.S. civil airports (through 1975) is shown in figure 36. Over the past 3 years (1973 to 1975), an average of seven air carrier airport runways per year have been given this antihydroplaning pavement surface treatment. Figure 34 shows that this surface is definitely superior in traction qualities over conventional ungrooved concrete and ranks with pavement grooving in this regard as reported in reference 19. PFC has a high storage volume to prevent runway flooding when rain first commences but does not have the free flowing drainage features common to grooved runways. Consequently (as discussed earlier in the paper), PFC surface treatments are not believed to be as effective as grooved pavements, especially those cut with diamond saws, in preventing runway flooding during sustained, high rainfall rate precipitation conditions.

A major problem that has been reported for FFC pavements is the difficulty of removing rubber from contaminated touchdown areas of the runway. AOCI (Airport Operators Council International) reports that the PFC surface at Johannesburg had to be replaced because rubber deposits could not be removed from the surface. A similar problem has been encountered at Denver Stapleton Airport where the rubber deposits could be removed only through the use of a flailing machine and high-pressure water-blast equipment. It should be stressed that the PFC surface treatments at U.S. airports have not been installed long enough at the present time to report realistically on the durability and maintainability of this type pavement surface.

Runway Rubber Deposits and Their Removal

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NASA, USAF, and FAA studies (tables 13 to 17) show that the most slippery runway segments are usually those located in aircraft touchdown areas which become covered with heavy rubber deposits. The reduced macro/microtexture of the pavement surface (fig. 37) resulting from rubber deposits makes the runway much more susceptible to dynamic and viscous hydroplaning during times of rain. The dramatic runway traction loss suffered as a consequence is illustrated by figure 38. Reference 11 points out that wheel spin-up at touchdown on the Roswell smooth concrete runway (SDR = 2.17 to 2.75 for DBV, B-737, and L-1011) required as much as 2 seconds. From a comparison of figures 13 and 38, the predicted aircraft tire friction coefficient μ_{skid} available to spin the tire up on the rubber coated ungrooved runway at MIA runway 9R/27L (SDR = 4.62) is found to be much less than at Roswell. Consequently, wheel spin-up times may take from 6 to 8 seconds on this wet, contaminated surface. As a consequence, pilots may apply wheel braking before the wheels are spun up with the result that the antiskid braking system fails to perform properly and poor braking, poor directional control along with reverted rubber skidding may occur for the aircraft. (See refs. 8 and 11.) Obviously, runway rubber deposits pose a distinct threat to the operational safety of aircraft during landings and take-offs in adverse weather. This paper has pointed out that ground vehicles which test pavements utilizing a constant speed technique cannot predict the runway slipperiness resulting to aircraft from this effect. Therefore, the DBV, which has a demonstrated capability to perform this measurement, should be the only device permitted to assess this runway condition. Only when test procedures have been changed and the devices correlated or calibrated satisfactorily with the DBV, should

other devices be allowed to measure the effects of rubber deposits on runway slipperiness for aircraft operation.

Review of the data contained in tables 13 to 17 and figures 37 and 38 indicates that grooved runways are much less affected by rubber deposits than ungrooved runways and may require less frequent cleaning. Several methods for cleaning runways of rubber deposits are available and discussed in reference 40. One of the most effective means is by high-pressure water blast as shown in figures 39 and 40.

Concluding Remarks

This paper has reviewed the runway slipperiness research performed in the United States and abroad over the time period 1968 to the present. This review suggests that this research has been extremely fruitful with the following tangible benefits resulting to the aviation community:

1. A better understanding of the hydroplaning phenomena

2. A method for predicting aircraft tire performance on wet runways from a ground-vehicle braking test

3. The runway rubber deposit problem has been defined as one of the most serious threats to aircraft operational safety during landings and takeoffs in adverse weather; at the same time, methods have been developed which can remove runway rubber deposits so that runway traction is effectively restored to uncontaminated levels

4. Pavement grooving has fulfilled its promise as a runway surface treatment that minimizes runway flooding during heavy rainstorms and produces nearly dry aircraft braking and cornering performance under wet runway conditions

5. Porous friction course surface treatments are nearly as effective as pavement grooving, but further research and time are required to assess the effects of rubber deposits (and removal), durability, and maintainability of this surface treatment

Finally, it is hoped that this report on the status of runway slipperiness research will stimulate the aviation community and the Federal Regulatory Agencies into a rapid implementation program to utilize the technological advances this research has produced and to improve airport runway safety.

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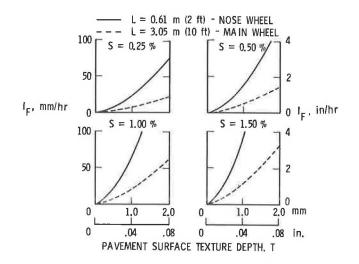


Figure 1. Rainfall rate required to flood tire path on conventional runway surfaces. Landings on center line.

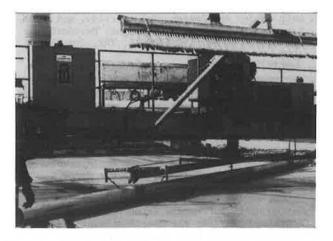


Figure 4. Space shuttle landing facility at KSC with slip-form paving equipment, leveling tube, and longitudinal broom.

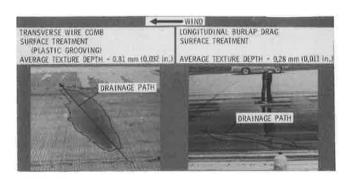


Figure 2. Water drainage from concrete runway at PHF. Water truck wetting; runway $6/24\,;$ wind from 60° at 10 knots.



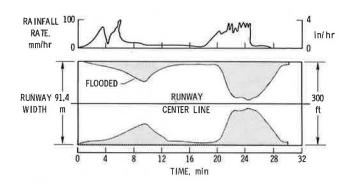
Figure 5. Space shuttle landing facility at KSC with pavement grooving machine (diamond blades).



Figure 3. Space shuttle landing facility at KSC.

 $29 \times 6 \times 6$ mm (1 1/8 \times 1/4 \times 1/4 in. TRANVERSE GROOVE PATTERN LONGITUDINAL BROOM FINISH ATD = 1.70 to 1.91 mm ATD = 0,38 to 0.64 mm (0,067 to 0,075 in.) (0.015 to 0.025 in.)

Figure 6. Concrete runway surface texture of space shuttle landing facility at KSC.



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Figure 7. Surface flooding on space shuttle grooved runway during thunderstorm 6/20/76.



Figure 10. B-737 tire reverted rubber skid patch after 1.8 km (6000 ft) locked-wheel skid on wet smooth concrete.

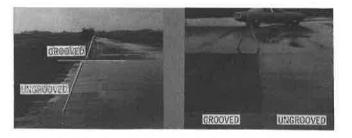


Figure 8. Water drainage from grooved and ungrooved asphalt. Grooving pattern, $38 \times 6 \times 6$ mm (1-1/2 × 1/4 × 1/4 in.).

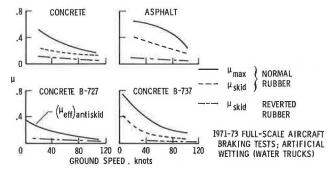


Figure 11. Aircraft flight test confirmation of reverted rubber hydroplaning 1965 NASA track; 32 × 8.8 aircraft tire; flooded runway.

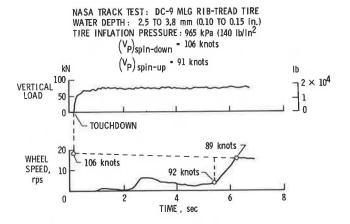


Figure 9. Delayed wheel spin-up at touchdown on flooded runway.

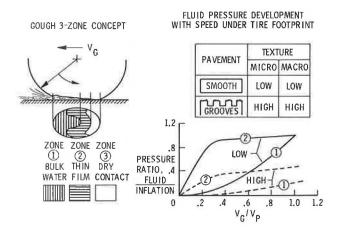


Figure 12. NASA model for combined viscous and dynamic tire hydroplaning.

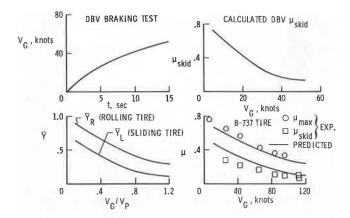


Figure 13. Prediction of aircraft tire friction coefficient from ground-vehicle braking test on a wet runway by NASA theory.

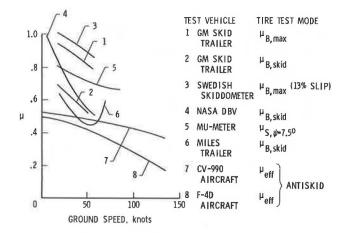


Figure 16. Aircraft/ground-vehicle correlation problem for wet and puddled grooved asphalt.

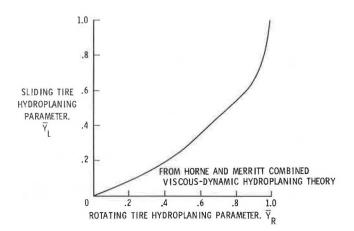


Figure 14. Empirically derived relationship between sliding $(\bar{\mathtt{Y}}_L)$ and rotating $(\bar{\mathtt{Y}}_R)$ tire hydroplaning parameters.

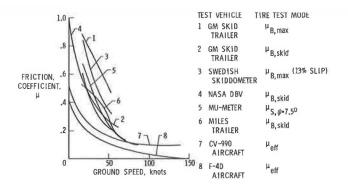


Figure 15. Aircraft/ground-vehicle correlation problem for wet and puddled smooth concrete surface.

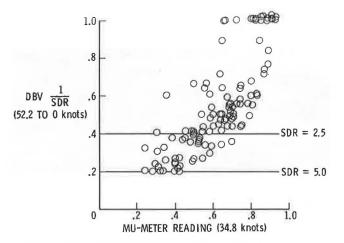


Figure 17. DBV/Mu-Meter relationship found by USAF tests (ref. 28).

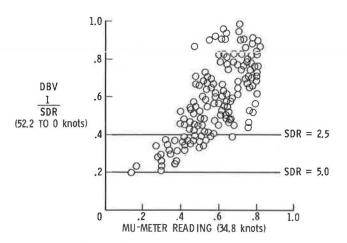


Figure 18.- DBV/Mu-Meter relationship found by FAA tests on 31 runways.

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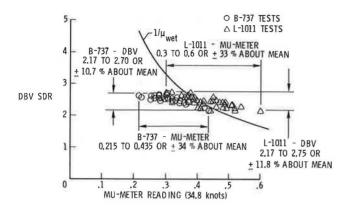


Figure 19. Comparison of NASA DBV with Mu-Meter.

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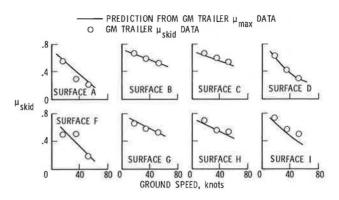


Figure 22. Prediction of GM trailer μ_{skid} from GM trailer μ_{max} data. ASTM smooth tread tire; data from reference 22.

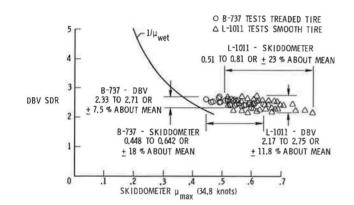


Figure 20. Comparison of NASA DBV with skiddometer.

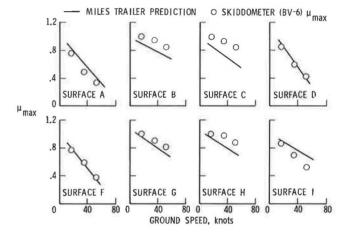


Figure 23. Prediction of skiddometer μ_{max} from Miles trailer μ_{skid} data. Data from references 21 and 22.

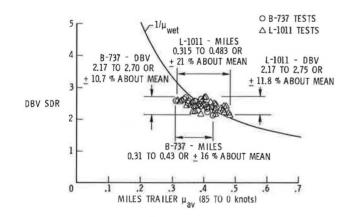


Figure 21. Comparison of NASA DBV with Miles trailer.

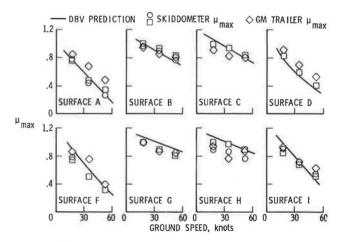


Figure 24. Prediction of skiddometer and GM trailer μ_{max} from DBV μ_{skid} data. Data from references 21, 22, and 30.

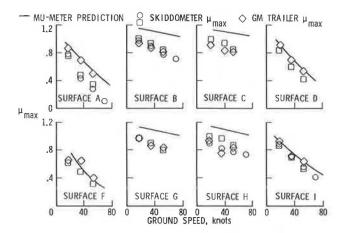


Figure 25. Prediction of skiddometer and GM trailer μ_{max} from Mu-Meter friction reading ($\psi = 7.5^{\circ}$). Data from references 21, 22, and 30.

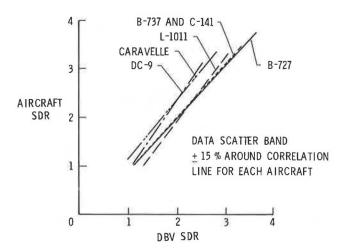


Figure 28. Aircraft/DBV correlation on wet runways for different jet transports.

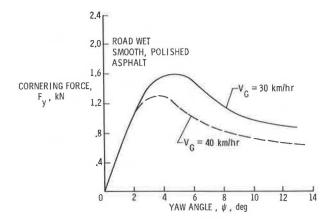


Figure 26. Effect of ground speed on corneringforce-yaw-angle relationships for 5.60-13 automobile tire. $F_z = 2.70 \text{ kN}$; p = 167 kPa; from reference 31.

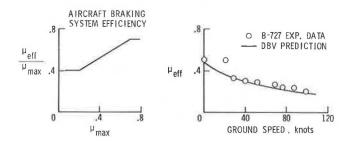


Figure 29. Prediction of aircraft braking performance on wet runway from DBV braking test. JFK runway 4R/22L; grooved concrete; water truck wetting.

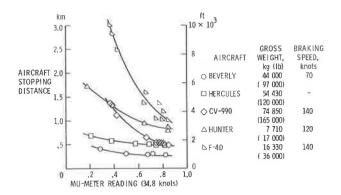


Figure 27. Mu-Meter correlation with aircraft stopping distances on wet surfaces.

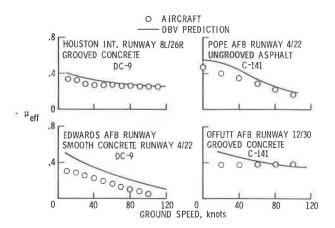


Figure 30. Prediction of aircraft braking performance on wet runways from DEV braking test for DC-9 and C-141 jet transports.

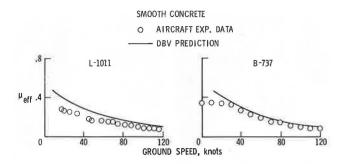


Figure 31. Prediction of aircraft braking performance on wet runway from DBV braking test for B-737 and L-1011 jet transports. Roswell runway 3/21; smooth concrete.

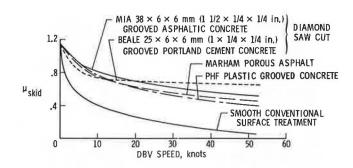
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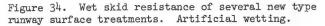
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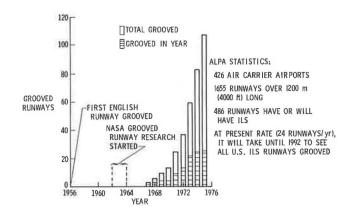


Figure 32. Number of grooved runways at U.S. air carrier airports.

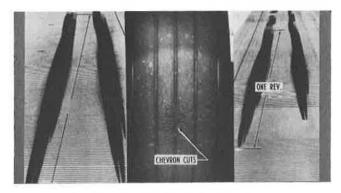


Figure 35. Tire damage from wheel spin-up at touch-down on dry grooved runway. Wallops grooved concrete; groove pattern, $25 \times 6 \times 6$ mm (1 × 1/4 × 1/4 in.); CV-990 jet transport MLG tire, size 41 × 15.0-18; p = 1102 kPa (160 lb/in²); V_G = 125 knots.

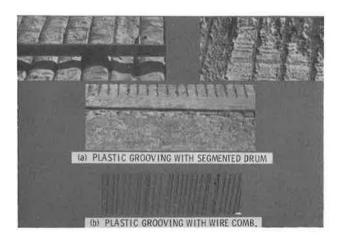


Figure 33. Examples of plastic grooving of Portland cement concrete.

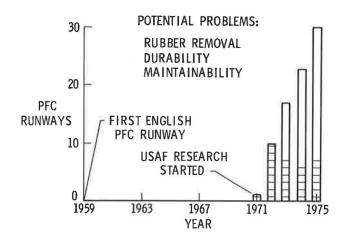


Figure 36. Number of porous friction course runways at U.S. air carrier airports.

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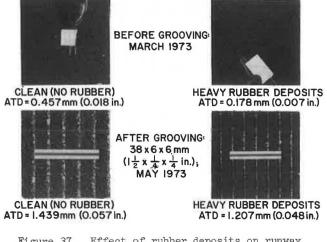


Figure 37. Effect of rubber deposits on runway surface texture.



BEFORE RUBBER REMOVAL MAY 15, 1975



AFTER RUBBER REMOVAL MAY30, 1975

Figure 39. Approach end of LAFB runway 25 before and after rubber removal by high-pressure water blast.

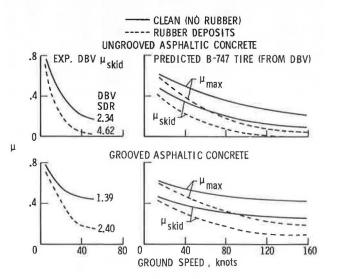


Figure 38. Effect of rubber deposits on runway traction before and after grooving.

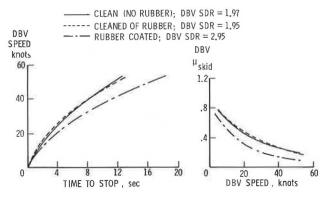


Figure 40. Effect of rubber removal by high-pressure water blast on runway traction. LAFB runway 25; May 1975.

Device			р
Device	^µ dry	kPa	1b/in ²
DBV (ASTM E-249 smooth tread tire)	1.15	165	24
DBV (ASTM E-524 smooth tread tire)	1.20	165	24
Mu-Meter	0.84	69	10
Miles trailer	1.15	138	20
Skiddometer model BV-6 (ASTM E-249 smooth tread tire)	1.15	165	24

Table 1. Tire characteristics of friction measuring devices

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Airport	Country	Runway	Surface	Grooving		ve pattern, ×W×D
Airport	country	Ruiway	buildee	technique	mm	in.
A (1956) - M	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
B (1957) - M	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
C (1960) - M	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
D (1960) - M	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
E (1960) - M	UK	N/A	PCC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
F (1961) - M	UK	N/A	AC	T-CS	25 × 3 × 3	$1 \times 1/8 \times 1/8$
Manchester (1961) - C	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
NASA LaRC (1964) - C	USA	Research track	AC PCC	T-DS L-DS	$\begin{array}{c} 25 \times 3 \times 3 \\ 25 \times 6 \times 6 \end{array}$	$1 \times 1/8 \times 1/8$ $1 \times 1/4 \times 1/4$
Manchester (1965) - C	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
Ubon (1966) - M	USA	N/A	PCC	T-DS	51 × 6 × 6 (Skip 610)	$2 \times 1/4 \times 1/4$ (Skip 24)
Udorn (1966) - M	USA	N/A	PCC	T-DS	51 × 6 × 6 (Skip 610) (25 × 3-9 × 3	2 × 1/4 × 1/4 (Skip 24) 1 × 1/8-3/8 × 1/8
NASA LaRC (1966) - C	USA	Research track	PCC	T-F T-DS	$ \begin{array}{c} 38 \times 3-9 \times 3 \\ 51 \times 3-9 \times 3 \\ 25 \times 3-9 \times 6 \\ 38 \times 3-9 \times 6 \\ 51 \times 3-9 \times 6 \end{array} $	$\begin{array}{c} 1 \ 1/2 \ \times \ 1/8 - 3/8 \ \times \ 1/8 \\ 2 \ \times \ 1/8 - 3/8 \ \times \ 1/8 \\ 1 \ \times \ 1/8 - 3/8 \ \times \ 1/4 \\ 1 \ 1/2 \ \times \ 1/8 - 3/8 \ \times \ 1/4 \\ 2 \ \times \ 1/8 - 3/8 \ \times \ 1/4 \end{array}$

Table 2. Grooved runways constructed during 1956-1966

Airport	Country Runway	Surface	Grooving	Groove pattern, $P \times W \times D$		
				technique	mm	in,
Bien Hoa - M	USA	N/A	PCC	T–DS	51 × 6 × 6 (Skip 610)	$2 \times 1/4 \times 1/4$ (Skip 24)
Birmingham - C	UK	N/A	AC	T-F	25 × 3 × 3	$1 \times 1/8 \times 1/8$
Beale AFB - M	USA	14/32	PCC	T-DS	25 × 6 × 6	$1 \times 1/4 \times 1/4$
John F. Kennedy - C	USA	4R/22L	PCC	T-DS	$38 \times 10-5 \times 3$	$1 \frac{1}{2} \times \frac{3}{8} - \frac{3}{16} \times \frac{1}{8}$
Kansas City Mun C	USA	18/36	PCC/AC	T-DS	25 × 3 × 6	$1 \times 1/8 \times 1/4$
NASA Wallops - C	USA	4/22	PCC/AC	T-DS	25 × 6 × 6	$1 \times 1/4 \times 1/4$
Washington Nat C	USA	18/36	AC	T-DS	25 × 3 × 3	$1 \times 1/8 \times 1/8$

Table 3. Grooved runways constructed during 1967

Airport	Country	Runway	Surface	Grooving technique		ve pattern, $\times W \times D$
				teennique	mm	in.
Atlanta Mun C Chicago-Midway - C Chicago-Midway - C Seymour-Johnson	USA USA USA	9R/27L 13R/31L 4R/22L	PCC PCC PCC	T-DS T-DS T-DS	$32 \times 10 - 3 \times 6$ $32 \times 6 \times 6$ $32 \times 6 \times 6$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
AFB – M	USA	8/26	PCC/AC	T-DS	51 × 6 × 6 (Skip 610)	$2 \times 1/4 \times 1/4$ (Skip 24)
Tempelhof (Ger.) - M	USA	9R/27L	AC	T-DS	38 × 10 × 10	1 1/2 × 3/8 × 3/8

Table 4. Grooved runways constructed during 1968

Airport	Country	Runway	Way I Surrace I	Grooving technique	Groove pattern, $P \times W \times D$		
			Lecinique	mm	in.		
Boston Logan - C Charleston (W.Va.) - C Chicago O'Hare - C Dallas Love Field - C Offutt AFB - M Wellington - C	USA USA USA USA USA New Zealand	N/A 5/23 9L/27R 13R/31L 12/30 N/A	AC PCC/AC PCC/AC PCC PCC AC	T-DS T-DS T-DS T-DS T-DS T-DS T-DS	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c} 1 \times 1/4 \times 1/4 \\ 1 \ 1/4 \times 1/4 \times 1/4 \\ 1 \ 1/8 \times 1/8 \end{array}$	

Table 5. Grooved runways constructed during 1969

Airport	Country	Country Runway		Grooving	Groove pattern, $P \times W \times D$		
• • • • • • • • • • • • • • • • • • •				technique	mm	in.	
Bankok Dallas Love Field - C Harry S. Truman - C Kadena - M Nashville Met C Nashville Met C Orly - C Port Hardy - C	Thailand USA USA USA USA France Canada	N/A 13L/31R 9/27 N/A 2L/20R 13/31 N/A N/A	PCC AC PCC/AC PCC/AC AC PCC/AC PCC AC	T-DS T-DS/CS T-DS/CS T-DS/CS T-DS/CS T-DS/CS T-DS T-DS	$51 \times 6 \times 6 38 \times 10 \times 6 38 \times 10 \times 6 32 \times 6 \times 6 51 \times 6 \times 6 32 \times 6 \times 6 N/A 25 \times 6 \times 6 10 + 10 + 10 + 10 + 10 + 10 + 10 + 10 +$	$\begin{array}{c} 2 \times 1/4 \times 1/4 \\ 1 \ 1/2 \times 3/8 \times 1/4 \\ 1 \ 1/2 \times 1/4 \times 1/4 \\ 1 \ 1/4 \times 1/4 \times 1/4 \\ 2 \times 1/4 \times 1/4 \\ 1 \ 1/4 \times 1/4 \times 1/4 \\ 1 \ 1/4 \times 1/4 \times 1/4 \\ N/A \\ 1 \ \times 1/4 \times 1/4 \end{array}$	
Shemya - M	USA	10/28	AC	T-DS	$32 \times 6 \times 6$	1 1/4 × 1/4 × 1/4	

Table	6.	Grooved	runways	constructed	during	1970
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Airport	Country	Runway	Surface	Grooving technique	Groove pattern, P × W × D			
					mm	in.		
Boston Logan - C	USA	4R/22L	AC	T-DS	57 × 6 × 6	$2 1/4 \times 1/4 \times 1/4$		
Chicago O'Hare - C	USA	4R/22L	PCC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$		
Houston Int C	USA	8L/26R	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$		
Kaitak - C	Hong Kong	13/31	PCC/AC	T-DS	$44 \times 6 \times 6$	$1 3/4 \times 1/4 \times 1/4$		
Kunsan - M	USA	17/35	PCC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$		
LaGuardia - C	USA	4/22	AC	T-DS	38 × 10-5 × 5	$1 \frac{1}{2} \times \frac{3}{8} - \frac{3}{16} \times \frac{3}{16}$		
LaGuardia - C	USA	13/31	AC	T-DS	38 × 10-5 × 5	$1 \frac{1}{2} \times \frac{3}{8} - \frac{3}{16} \times \frac{3}{16}$		
Memphis Int C	USA	17R/35L	PCC	T-PGWB	N/A	N/A		
Newark - C	USA	4L/22R	AC	T-DS	$38 \times 10-5 \times 5$	$1 1/2 \times 3/8 - 3/16 \times 3/16$		
San Diego Lindberg - C	USA	9/27	PCC	T-DS	$25 \times 6 \times 6$	$1 \times 1/4 \times 1/4$		
Springfield (I11.) - C	USA	4/22	PCC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$		
Tampa Int C	USA	18L/36R	PCC	T-PGWB	N/A	N/A		

Table 7. Grooved runways constructed during 1971

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Airport	Country Runway		Surface	Grooving technique	Groove pattern, $P \times W \times D$			
				technique	mm	in.		
Baton Rouge - C	USA	4/22	PCC	T-PG	51 × 6 × 6	$2 \times 1/4 \times 1/4$		
Boston Logan - C	USA	4R/22L	AC	T-DS	57 × 8 × 6	$2 1/4 \times 5/16 \times 1/4$		
Cincinnati - C	USA	18/36	AC	T-DS	$38 \times 6 \times 6$	$1 1/2 \times 1/4 \times 1/4$		
Cincinnati - C	USA	9R/27L	PCC	T-DS	$38 \times 6 \times 6$	$1 1/2 \times 1/4 \times 1/4$		
Denver Stapleton - C	USA	17L/35R	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$		
Detroit Metro C	USA	3L/21R	PCC	T-DS	32 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$		
Minneapolis - C	USA	4/22	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$		
Oklahoma City - C	USA	17R/35L	PCC	T-PG	25 × 13 × 13	$1 \times 1/2 \times 1/2$		
Omaha Eppley Field - C	USA	14R/32L	AC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$		
Osan - M	USA	9/27	PCC	T-DS	32 × 6 × 6	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$		
Plattsburg - M	USA	17/35	PCC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$		
Shaw - M	USA	4L/22R	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$		
		angle and the first		9) 9)	(Skip 610)	(Skip 24)		
Springfield (Mo.) - C	USA	1/19	PCC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$		
St. Paul Holman - C	USA	12/30	AC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$		
Waterloo Mun C	USA	12/30	PCC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$		
Washington Nat C	USA	18/36	AC	T-DS	32 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$		

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Table 8. Grooved runways constructed during 1972

Airport	Country	Runway	Surface	Grooving		ve pattern, × ₩ × D
				technique	mm	in.
Allentown - C	USA	6/24	AC	T-DS	32 × 6 × 6	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Atlanta Int C	USA	9R/27L	PCC	T-PGWC	N/A	N/A
Baltimore Int C	USA	10/28	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Baltimore Int C	USA	15/33	AC	T-DS	32 × 6 × 6	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Charles DeGaulle - C	France	N/A	PCC	T-DS	N/A	N/A
Clarksburg - C	USA	3/21	AC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$
Cleveland Hopkins - C	USA	5R/23L	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Dallas/Ft. Worth - C	USA	17L/35R	PCC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Dallas/Ft. Worth - C	USA	17R/35L	PCC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Dallas/Ft. Worth - C	USA	13L/31R	PCC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Gainsville Mun C	USA	10/28	AC	T-DS	$38 \times 6 \times 6$	$1 1/2 \times 1/4 \times 1/4$
Griffiss - M	USA	15/33	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$
Huntington - C	USA	12/30	AC	T-DS	32 × 6 × 6	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Jacksonville Int C	USA	7/25	AC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$
Lafayette (Ind.) - C	USA	10/28	AC	T-DS	32 × 6 × 6	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
LaGuardia - C	USA	13/31	AC	T-DS	38 × 10-5 × 5	$1 1/2 \times 3/8 - 3/16 \times 3/10$
Miami Int C	USA	9L/27R	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Miami Int C	USA	9R/27L	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Patrick Henry Field - C	USA	6/24	PCC	T-PGWC	13 × 3 × 3	$1/2 \times 1/8 \times 1/8$
Peoria (Ill.) - C	USA	12/30	AC	T-DS	51-76 × 6 × 6	$2-3 \times 1/4 \times 1/4$
Savannah - C	USA	18/36	PCC	T-PGWC	N/A	N/A
South Bend - C	USA	9/27	AC	T-DS	32 × 6 × 6	1 1/4 × 1/4 × 1/4
St. Louis Lambert - C	USA	6/24	PCC	T-DS	32 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$
Vance - M	USA	17R/35L	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$
Williamsport - C	USA	9/27	AC	T-DS	32 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$

Table 9. Grooved runways constructed during 1973

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Airport	Country	Runway	Surface	Grooving		ove pattern, $P \times W \times D$
				technique	mm	in.
Albany (N.Y.) - C	USA	10/28	AC	T-DS	32 × 6 × 6	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Allentown - C Bagotville - M	USA	13/31	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
5	Canada	11/29	PCC	T-PGWC	N/A	N/A
Bangor - C	USA	15/33	PCC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Cedar Rapids - C	USA	8/26	AC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$
Cedar Rapids - C	USA	13/31	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Chattanooga - C	USA	2R/20L	AC	T-DS	$38 \times 6 \times 6$	$1 1/2 \times 1/4 \times 1/4$
Chicago O'Hare - C	USA	14L/32R	AC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$
Chicago O'Hare - C	USA	14R/32L	AC	T-DS	$32 \times 6 \times 6$	$1 1/4 \times 1/4 \times 1/4$
Chicago O'Hare - C	USA	9R/27L	AC	T-DS	32 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$
Cleveland Hopkins - C	USA	10L/28R	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
England - M	USA	14/32	PCC	T-DS	51 × 6 × 6	$2 \times 1/4 \times 1/4$
					(Skip 610)	(Skip 24)
Elsworth - M	USA	12/30	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Harry S. Truman - C	USA	9/27	AC	T-DS	51 × 10 × 6	$2 \times 3/8 \times 1/4$
Jacksonville Int C	USA	7/25	AC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$
John F. Kennedy - C	USA	13L/31R	AC	T-DS	38 × 10-5 × 5	$1 1/2 \times 3/8 - 3/16 \times 3/16$
John F. Kennedy - C	USA	4L/22R	PCC/AC	T-DS	38 × 10-5 × 5	$1 1/2 \times 3/8 - 3/16 \times 3/16$
Lawton - C	USA	17/35	PCC	T-PG	51 × 6 × 3	$2 \times 1/4 \times 1/8$
Los Angeles Int C	USA	6R/24L	PCC/AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$
Louisville - C	USA	1/19	PCC	T-PGWB	N/A	N/A
Memphis Int C	USA	17L/35R	PCC	T-PGWB	N/A	N/A
Minneapolis - C	USA	11R/29L	PCC/AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Newark - C	USA	4R/22L	AC	T-DS	$38 \times 10-5 \times 5$	$1 1/2 \times 3/8 - 3/16 \times 3/16$
Patrick Henry Field - C	USA	2/20	PCC	T-PGWC	13 × 3 × 3	$1/2 \times 1/8 \times 1/8$
Pittsburg - C	USA	10L/28R	PCC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Ponca City - C	USA	17/35	PCC	T-PG	$51 \times 6 \times 3$	$2 \times 1/4 \times 1/8$
Washington Nat C	USA	18/36	AC	T-DS	32 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$

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Table 10. Grooved runways constructed during 1974

Airport	Country	Runway	Surface	Grooving	Gro	ove pattern, $P \times W \times D$
				technique -	mm	in.
Arlanda - C	Sweden	N/A	PCC	N/A	25 × 3 × 3	$1 \times 1/8 \times 1/8$
Beaumont - C	USA	11/29	PCC	T-PG	51 × 6 × 6	$2 \times 1/4 \times 1/4$
Boston Logan - C	USA	4L/22R	AC	T-DS	$57 \times 6 \times 6$	$2 1/4 \times 1/4 \times 1/4$
Boston Logan - C	USA	15R/33L	AC	T-DS	57 × 6 × 6	$2 1/4 \times 1/4 \times 1/4$
Cannon - M	USA	3/21	PCC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$
					(Skip 610)	(Skip 24)
Charlotte - C	USA	5/23	PCC/AC	T-DS	$44 \times 6 \times 6$	$1 3/4 \times 1/4 \times 1/4$
Chicago O'Hare - C	USA	9L/27R	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Chicago O'Hare - C	USA	4L/22R	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Denver Stapleton - C	USA	17L/35R	PCC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$
Des Moines Mun C	USA	12L/30R	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Dunedin - C	New	N/A	N/A	N/A	N/A	N/A
Junearin o	Zealand	11/21	M/A	N/A	N/A	N/A
Elmíra - C	USA	10/28	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Erie - C	USA	6/24	AC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$
Fort Lauderdale - C	USA	9L/27R	AC	T-DS	$38 \times 6 \times 6$	$1 \frac{1}{2} \times \frac{1}{4} \times \frac{1}{4}$
Grand Forks - M	USA	17/35	PCC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$
Houston Int C	USA	14/32	PCC	T-PG	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$ $2 \times 1/4 \times 1/4$
Invercargill - C	New	N/A	N/A-	N/A	N/A	N/A
invercargini 6	Zealand	N/A	N/A	N/A	N/ A	N/A
Kansas City Int C	USA	9/27	PCC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Kansas City Int C	USA	1/19	PCC/AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$ $1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Kincheloe - M	USA	15/33	PCC	T-DS	$51 \times 6 \times 6$	$2 \times 1/4 \times 1/4$
Knoxville - C	USA	4L/22R	PCC	T-PGWB	N/A	N/A
Lubbock Int C	USA	8/26	PCC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Monroe (La.) - C	USA	4/22	PCC	T-PG	$13 \times 6 \times 3$	$1/2 \times 1/4 \times 1/4$
New Haven - C	USA	2/20	AC	T-DS	$48 \times 6 \times 6$	$1/2 \times 1/4 \times 1/8$ 1 7/8 × 1/4 × 1/4
Pittsburg - C	USA	14/32	PCC/AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{14} \times \frac{1}{4} \times \frac{1}{4}$
Pittsburg - C	USA	10R/28L	PCC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$ $1 \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
San Antonio - C	USA	12R/30L	PCC	T-PG	$52 \times 6 \times 6$ 51 × 6 × 6	$1 \frac{1}{2} \times \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Tallahassee - C	USA	18/36	AC	T-DS	$44 \times 6 \times 6$	$2 \times 1/4 \times 1/4$ 1 3/4 × 1/4 × 1/4
Tampa - C	USA	187/36L	AC	T-DS	$44 \times 6 \times 6$ $44 \times 6 \times 6$	$1 3/4 \times 1/4 \times 1/4$ 1 3/4 × 1/4 × 1/4
Washington Nat C	USA	15/33	AC	T-DS	$32 \times 6 \times 6$	$1 \frac{1}{1} \frac{3}{4} \times \frac{1}{4} \times \frac{1}{4} \times \frac{1}{4}$
Wilkes-Barre - C	USA	4/22	AC	T-DS T-DS	$32 \times 6 \times 6$ 38 × 6 × 6	$1 1/4 \times 1/4 \times 1/4$ $1 1/2 \times 1/4 \times 1/4$
Victoria Int C	Canada	4/22 N/A	N/A	N/A	38 × 6 × 6 N/A	1 1/2 × 1/4 × 1/4 N/A
Zurich - C	Switzerland	N/A N/A	N/A N/A		N/A N/A	N/A N/A
Adriell - C	Switzeriand	M/A	N/A	N/A	N/A	IN/ 25

Table 11. Grooved runways constructed during 1975

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Airport	Country	Runway	Surface	Grooving technique	Groove pattern, $P \times W \times D$			
				cecimique	mm	in.		
Albany County - C	USA	N/A	AC	T-DS	$32 \times 6 \times 6$	1 1/4 × 1/4 × 1/4		
Boston Logan - C	USA	N/A	AC	T-DS	57 × 6 × 6	$2 1/4 \times 1/4 \times 1/4$		
Cumberland (Md.) - C	USA	N/A	N/A	T-DS	N/A	N/A		
Jackson County (W.Va.) - C	USA	N/A	AC	T-DS	32 × 6 × 6	1 1/4 × 1/4 × 1/4		
Lihue (H.I.) - C	USA	N/A	AC	T-DS	44 × 6 × 6	$1 3/4 \times 1/4 \times 1/4$		
NASA Kennedy - C	USA		PCC	T-DS	$29 \times 6 \times 6$	$1 \frac{1}{8} \times \frac{1}{4} \times \frac{1}{4}$		
Raleigh Heights (W.Va.) - C	USA	N/A	PCC/AC	T-DS	32 × 6 × 6	1 1/4 × 1/4 × 1/4		
Wood County (W.Va.) - C	USA	N/A	AC	T-DS	38 × 6 × 6	$1 1/2 \times 1/4 \times 1/4$		

Table 12. Grooved runways constructed during 1976

				uchdown bber dep			Traffic no rub		ľ	Intraffic no rubl	
Airfield	Runway	Surface	SDR	A	TD	SDR		ATD	SDR	l	ATD
			(a)	mm	in.	(a)	mm	in.	(a)	mm	in.
Travis	21L	PCC	5.79	0.3759	0.0148	2.28	0.9677	0.0381			
Fairchild	23	PCC	4.75	.1092	.0043	1.97	.4318	.0170	1.97	0.2769	0.0109
Castle	30	AC	4.60	.1448	.0057	2.00			1.59	.8306	.0327
Loring	01	AC	4.58	.1499	.0059	1.99	.3632	.0143			
Travis	21R	AC	4.01	.3632	.0143	2.71	.4140	.0163	2.18	.5537	.0218
McGuire	24	AC	3.92	.1575	.0062	1.93	.3073	.0121	1.33		
Torrejon	23	AC	3.85	.1626	.0064	1.85	1.1633	.0458	1.50	.6452	.0254
Mather	22L	PCC/AC	3.75	.2083	.0082	1.86	.4140	.0163			
Blytheville	17	PCC	3.73			2.45			1.57		
Dover	01	PCC/AC	3.62			1.74			1.47		
Scott	31	AC	3.61			1.83			1.47		
Robbins	32	PCC	3.59	.2896	.0114	2.01	.4928	.0194			
Cannon	21	PCC/GPCC	3.59			1.74			1.43		
Rickenbacker	23L	PCC	3.40	.2769	.0109	2.04	.4851	.0190	1.86	.5055	.0199
Homestead	05	PCC	3.37	.2235	.0088	1.92	.7061	.0278	2.17	.4140	.0163
Grissom	22	AC	3.23	.1041	.0041	1.66	.5055	.0199	1.60	.5283	.0208
Charleston	15	AC/PCC	3.21	.2159	.0085	2.55			2.21	.3302	.0130
Zaragosa	31R	AC	2.93	.2591	.0102	1.31	.5817	.0229	1.32	.5537	.0218
Mather	22R	AC	2.90	.2083	.0082	2.18	.4140	.0163	1.67		2000
Andrews	01L	PCC	2.89	.4064	.0160	2.14	.5588	.0220	2.28	.9398	.0370
Charleston	21	AC	2.79	.3327	.0131	1.88	.5817	.0229			
Shaw	4L	PCC/GPCC/AC	2.77	.3429	.0135	1.79	.7264	.0286	1.52	.4851	.0191
McConne1	18R	AC	2.77		وتستعد	2.03		1000			
Hector	35	PCC	272			1.95	.6121	.0241	1.89		
Dover	31	AC	2.66			1.89			1.28		
Columbus	13L	PCC/AC	2.62	.4851	.0191	1.80	.4851	.0191	1.71	.5537	,0218
Glasgow	28	PCC	2.61	.3632	.0143	2.11	.2464	.0097	2.37	.1727	.0068
Andrews	O1R	PCC/AC	2.60	.3302	.0130	1.73	.635	.025	1.82	.686	.027
England	14	PCC	2.54			2.66	.5055	.0199			
Aviano	05	AC	2.51	.889	.035	1.73	1.168	.046	1.84	.965	.038
R. Gebaur	36	PCC/AC	2.50			2.22			2.29		
Vance	17R	PCC/AC/GPCC	2.50			1.50			1.53		
Soesterberg	28	AC	2.42			2.29			1.57		
Columbus	13R	PCC	2.40		-	2.28	- Harrison				
England	18	PCC/AC	2.39	.6452	.0254	2.57	.6375	.0251	2.40		
Moody	18R	PCC/AC	2.38	.4851	.0191	1.48	1.1633	.0458	1.32	1.1633	.0458
Zweibrucken	03	AC	2.34	.5283	.0208	1.35	.8941	.0352	1.16	.7264	.0286
Bentwaters	25	PCC/AC	2.33	.4851	.0191	1.44	1.1633	.0458	1.57	.6121	.0241
Moody	18L	PCC/AC	2.32	.3073	.0121	1.66	.5283	.0208	1.45	.6121	.0241
Craig	32L	PCC/AC	2.27	.4318	.0170	1.70	.3327	.0131	1.42	1.3462	.0530
Rickenbacker	23R	AC	2.26			1.94					
Vance	17C	PCC/AC	2.25	.1448	.0057	1.45	.8941	.0352	1.52	1000	
Columbus	13C	PCC/AC	2.22			1.90			2.13		
Woodbridge	27	AC	2.22			1.53			2.01		
Niagara Falls	28	AC	2.12	.1651	.0065	1.80	.4851	.0191	1,28	.6121	.0241
Vance	17L	PCC	2.10			2.09	.4851	.0191			
McConnel	18L	AC	2.03			1.73			1.89		
McGuire	36	PCC/AC	2.00	.1575	.0062	1.66	. 3023	.0121	1.36		
Myrtle Beach	17	PCC/AC	2.00	.4013	.0158	1.57	.5283	.0208	1.52	.6452	.0254
Cannon	30	PCC/AC	2.00			1.65			1.81	.0452	
Shaw	04R	PCC/PGWC/PCC	1.99	.3150	.0124	1.13	1.5570	.0613	1.38		
Erding	26	PCC	1.93	.2184	.0086	2.04	.4851	.0191	1.73	.4470	.0176
Hurlburt	35	PCC/AC	1.89	.5055	.0199	1.92	.6833	.0269	1.34	.8306	.0327

^aDBV SDR 3 minutes after wetting.

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Table 13. DBV SDR and NASA grease test ATD obtained on runways evaluated July 1973 to December 1974 by AFCEC [From reference 28]

		Rubb	er-coated	touchdow	n areas		ficked, rubber		fficked, rubber
Airfield	Runway	Рт	imary	Sec	ondary	(whee	1 paths)	(runw	ay edge)
		SDR (a)	Surface	SDR (a)	Surface	SDR (a)	Surface	SDR (a)	Surface
Palmdale	07/25	6.12	PCC	2,55	PCC	2.31	PCC		PCC/AC
March	13/31	5.19	PCC	2.46	PCC	2.21	PCC		AC
Barksdale	14/32	4.73	AC	3.70	AC	1.84	AC	1.40	AC
Norton	05/23	4.58	PCC	2.75	PCC	2.19	PCC	2.40	PCC
bWebb	17L/35R	2.95	PCC/AC	1.51	PCC/AC	4.51	AC		AC
Dyess	16/34	3.52	PCC	4.46	PCC	2.61	PCC		AC
Carswell	17/35	3.78	AC/PCC	4.11	PCC	2.36	AC/PCC	1.32	AC
Elmendorf	05/23	3.53	AC	1.92	AC	2.95	AC	1.52	AC
Reese	17R/35L	3.03	PCC	1.85	PCC/AC	1.80	PCC/AC	1.72	AC
Davis Monthan	12/30	2.98	AC	2,50	PCC	1.54	AC	1.39	AC
Palmdale	04/22	2.88	AC	2.43	AC	1.82	AC	2.05	AC
b _{Webb}	17R/35L	2.82	PCC/AC	2.65	PCC/AC	2.69	AC	and in	AC
Laughlin	13C/31C	2.70	PCC/AC	1.88	PCC/AC	1.69	AC	1.75	AC
Randolph	14L/32R	2.65	PCC	2.16	PCC	2.05	PCC	2.27	PCC
Yokota	18/36	2.61	PCC	1.95	PCC	1.91	PCC	1,94	PCC
Reese	17C/35C	2.37	AC	2.59	AC	2.15	AC	2.06	AC
Williams	12L/30R	2.52	PCC/AC	1.57	AC	1,68	AC	1,65	AC
^C Williams	12C/30C	2.39	PCC	*******			20-00-01-01		
Williams	12R/30L	2.36	PCC	2.16	PCC	2.22	PCC	2.03	PCC
Laughlin	13L/31L	2.15	PCC/AC	2.31	PCC/AC	1.35	AC		AC
Elmendorf	15/33	2.21	AC	1.86	AC	2.05	AC/PCC		AC
Laughlin	13R/31L	1.87	AC	2.20	AC	1.56	AC		AC
Randolph	14R/32L	2.13	PCC/AC	1.90	PCC	1.48	PCC/AC	1.39	PCC/AC
dVandenberg	12/30	1.59	AC	1.54	AC	1.60	AC	1.32	AC
Reese	17L/35R		PCC/AC		PCC/AC	1.39	AC	-	AC

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^aAverage DBV SDR 3 minutes after wetting.

 $^{\rm b}{\rm Asphalt}$ emulsion diluted with water applied to asphaltic concrete.

^CRunway under construction.

d_{New} runway surface.

Table 14. DEV SDR obtained on runways evaluated January to June 1975 by AFCEC [From reference 38]

Airport	Runway		Touchdo rubber	Trafficked, no rubber (wheel path)			
		SDR (a)	Surface	SDR (a)	Surface	SDR (a)	Surface
St. Louis Int.	12R/30L 6/24 12L/30R 17/35	4.79 2.48 1.93 1.77	12R:AC 24:PCC 30R:PCC 17:PCC	3.51 2.13 1.35 1.63	30L:AC 6:PCC 12L:WCPCC 35:PCC	2.90 1.85 1.81 1.79	AC PCC PCC PCC
Miami Int.	9R/27L 9L/27R 12/30 17/35	4.44 2.88 2.01 1.35	9R:AC 9L:AC 12:AC 17:AC	2.88 1.98 1.75 1.32	27L:AC 27R:AC 30:AC 35:AC	1.81 1.72 1.56 1.33	AC AC AC AC
Memphis Int.	17L/35R 9/27 b17R/35L 3/21	$ \begin{array}{r} 3.82 \\ 1.83 \\ \overline{} \\ 1.18 \end{array} $	17:PCC 27:AC 3:AC	3.51 1.58 1.16	35R:PCC 9:AC 21:AC	2.44 1.32 1.47 1.17	PCC AC PCC AC
New Orleans Int.	10/28 1/19 5/23	3.76 3.22 1.22	10:PCC 19:PCC 23:AC	2.22 3.03	28:AC 1:AC 5:AC	2,26 2,17 1.32	PCC PCC AC
Atlanta W. B. Hartsfield	9L/27R 15/33 9R/27L 3/21	2.88 2.21 2.09 1.69	9L:WCPCC 33:AC 27L:GPCC 21:AC	2.26 1.72 1.24 1.52	27R:WCPCC 15:AC 9R:GPCC 3:AC	1.38 1.50 1.12 1.36	WCPCC AC GPCC AC
Jacksonville Int,	7/25 13/31	2.77 2.65	7:PCC 31:PCC	2,53 2,33	25:PCC 13:PCC	2.12 1.97	PCC PCC
Greater Cincinnati	18/36 9R/27L 9L/27R	2.45 2.38 1.30	36:AC 27L:AC 9L:PCC	1.93 2.09 1.15	18:AC 9R:AC 27R:PCC	1.73 1.77 1.25	AC AC PCC
Charlotte Douglas	18/36 5/23	2.32 1.81	36:AC 5:AC	1.39 1.38	18:AC 23:AC	1.46	AC AC
Nashville Int.	13/31 2L/20R 2R/20L	2.12 2.08 1.30	31:AC 20R:GAC 20L:AC	1.71 1.82	13:AC 2L:GAC 2R:AC	1.69 2.04 1.24	AC GAC AC
Charleston Kanawha	5/23 14/32	1.33 1.20	23:GPCC 32:AC	1.10	5:GPCC 14:AC	1.09 1.16	GPCC AC

^aAverage DBV SDR.

^bNew surface; under construction.

Table 15. DBV SDR obtained at 10 civil airports evaluated November 1971 to April 1972 by FAA [From reference 29]

No.

Airport	Date	Runway	Rubber	SI	DR	Surface;	Groove pattern;	Source
	tested		deposits	DBV	Aircraft	date installed	date installed	
Cannon AFB	11/73	3/21	Heavy None	^a 3.59 to 2.46 1.74	I	305 m (1000 ft) PCC, 2438 m (8000 ft) GPCC, 305 m (1000 ft) PCC; date unknown	51 × 6 × 6 mm (2 × 1/4 × 1/4 in.), groove 610 mm (2 ft) skip 610 mm (2 ft); 1973	Reference 8
Shaw AFB	7/74	4L/27R	Med-1t None	2.77 to 1.97 1.79	11	305 m (1000 ft) PCC, 1367 m (3500 ft) GPCC, 1370 m (4500 ft) GAC, 305 m (1000 ft) PCC; date unknown	51 × 6 × 6 mm (2 × 1/4 × 1/4 in.), groove 610 mm (2 ft) skip 610 mm (2 ft); 1971	Reference 8
Vance AFB	12/73	17R/35L	Lt-med None	^a 2.50 ^a 1.50	-	457 m (1500 ft) PCC, 853 m (2800 ft) GPCC, 1036 m (3400 ft) AC; date unknown	51 × 6 × 6 mm (2 × 1/4 × 1/4 in.); 1973	Reference 8
Houston Int.	^b 6/70	8L/26R	Heavy- med	3.46 to 2.94	-	PCC; date unknown	Ungrooved	Unpublished
	10/71		Lt- none	2.08 to 2.52	^c 1.91 to 2.52		Ungrooved	
	2/25/71		None Heavy	1.13 to 1.44 2.27 to 2.43	^d 1.10 to 1.53 -		51 × 6 × 6 mm (2 × 1/4 × 1/4 in.); 2/24/71	Unpublished
Miami Int.	3/73	9R/27L 9L/27R	Heavy None Heavy- med	4.62 to 3.51 2.43 3.16 to 2.38	E E T	AC overlay; 11/72	Ungrooved	Unpublished
	5/73	9R/27L 9R/27L 9L/27R	Heavy- lt None None	2.42 to 1.51 1.51 1.22			38 × 10 × 6 mm (1 1/2 × 1/4 × 1/4 in.); ^e 1973	Unpublished
John F. Kennedy	7/69 7/69 10/71	4R/22L	None Heavy Lt- none	1.75 2.20 1.47 to 1.80	f _{1.57} 1.86 ^c 1.50 to 1.67	PCC; 1959	38 × 10-5 × 3 mm (1 3/8 × 3/8-3/16 × 1/8 in.); 1967	Reference l Unpublished
Atlanta Int.	11/71	9R/27L	Heavy- med None	2.09 to 1.24	-	PCC; date unknown	32 × 10-3 × 6 mm (1 1/4 × 3/8-1/8 × 1/4 in.); 1969	Reference 1
Nashville Int.	4/72	2L/20R	Lt None	2.08 to 1.82 2.04		AC; date unknown	32 × 6 × 6 mm (1 1/4 × 1/4 × 1/4 in.); 1970	Reference 1
Harry S. Truman	6/70	9/27	Heavy None Heavy None	2.28 1.40 1.69 1.18	-	AC; date unknown	Ungrooved 38 × 6 × 6 mm (1 1/2 × 1/4 × 1/4 in.); 4/70	Unpublished
Seymour-Johnson AFB	7/69	8/26	None Heavy- lt	1.35 1.50	^f 1.38 1.47	PCC; 1960	51 × 6 × 6 mm (2 1/4 × 1/4 × 1/4 in.), groove 610 mm (2 ft) skip 610 mm (2 ft); 1968	Reference 1

 $^{\mathrm{a}}_{\mathrm{DBV}}$ test area contained both grooved and ungrooved pavements.

^bRubber removed after test.

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 $^{\rm e}_{\rm 9L/27R}$ being grooved at time of test.

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Table 16. DBV and aircraft SDR obtained on transverse grooved runway surfaces with and without rubber contamination [From reference 39]

1 days and	D	Surface		Couchdown area cubber deposit	, S	Traffic	ked, no rub	ber
Airport	Runway		DBV SDI		TD	DBV SDR	A	TD
		(a)	257 051	mm	in.		mm	in.
Allentown	13 31 6 24		.0 1.07 1.29 2.07 1.71	1.56 1.49 1.52 1.60	0.061 .059 .060 .063	1.08 1.08 1.66 1.66	1.48 1.48 1.48 1.48	0.058 .058 .058 .058
Akron-Canton	1 19	AC 1.	.4 2.71 2.19	0.229	0.009	2.01	0.305	0.012
	5 23	AC 1	.4 1.32	.254 .330	.010 .013	1.47	.330	.013
Boston Logan	4L 22R 4R 22L 15R 33L	GAC 1	.0 1.12 1.00 .0 1.62 1.75 .0 1.39 2.50			0.90 .90 1.96 1.96 2.00 2.00		
Buffalo	5 23 ^b 14 ^b 32	AC 1	.0 2.06 2.68	0.838	0.033 .043	2.30 2.30	0.864 .864	0.034
Burlington	15 33	AC 1	.0 2.09 2.04	0.559	0.022	1.75	0.584	0.023
Charleston,W.Va.	5 23 ^b 14 ^b 32	GPCC 0	.8 1.27 1.42	0.211 .165	0.008 .007	1.35	0.221	0.009
	18 36	GAC 1	.5 1.51		0.053	2.05	1.016	0.040
Cincinnati	9R 27L	GPCC 1	.5 1.34 1.78	.838 1.270	.033 .050	1.38	1.194	0.047
Cleveland	5R 23L	GAC 1	.3 1.7 2.4	0.737	0.029	2.14	1.092	0.043
	10L 28R	GAC 0	.8 1.30	1.118	.044	1.57	1.702	.063
Detroit	3L 21R	GPCC 1	.0 1.58 1.65		0.050	1.68	1.270	0.050
	9 27	PCC/AC 1	.0 2.10 2.77	.432	.017	1.91	.508	.020
Dulles	1L 19R	PCC 1	.0 3.11 3.90		0.006	2.98	0.254	0.010
	1R 19L	PCC 1	.0 4.48 3.12		.009	3.13	.254	.010
Ft. Wayne	4 22	PCC/AC 0	.9 2.00 2.05		0.013	1.55	0.533	0.02
	9 27	PCC/AC N	/A 1.77 1.81		.040	1.80	. 508	. 020
Grand Rapids	8R 26L	PCC/AC 1	.5 2.36		0.010	1.57	0.127	0.00
Madison	18 36	PCC 1	.5 1.68		0.027 .044	1.43	1,016	0.04
	13 31	AC 1	.5 1.62		.015	1.54	.127	.00

 $^{\rm a}{\rm Number}$ on right of column represents the runway transverse slope in percent. $^{\rm b}{\rm Under}$ construction,

Table 17. DBV SDR and NASA grease test ATD obtained on runways evaluated by the FAA trial application — runway friction calibration and pilot information program during August and September 1975

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		Surface			chdown area, per deposits		Traffi	cked, no rub	ober
Airport	Runway			DBV SDR	AT	D		A	TD
		(a)	DDV SDR	mm	in.	DBV SDR	A mm 0.432 0.508 .127 1.397 1.270 0.279 1.600 1.600 0.737 .279 0.356	in.
Milwaukee	^b 1L ^b 19R								
	7R 25L	PCC	1.0	1.67 3.02	0.838	0.033	1.56	0.432	0.017
Moline	9 27	AC	1.0	2.34 2.77	0.508	0.020 .004	2.65	0.508	0.020
northe	12 30	PCC	1.0	2.66 2.91	.216 .152	.009	2.49	.127	.00
Peoria	30 12	GAC	1.25	1.36 1.52	0.965 .991	0.038 .039	1.16	1.397	0.05
	4 22	AC	1.0	1.14 1.44	.635 .305	.025	1.43	1.270	.050
Philadelphia	9R 27L	AC	1.0	4.99 3.57	0.127 .127	0.005	2.47	0.279	0.01
Pittsburg	10R 28L	GAC	1.0	2.15 2.54	1.549 1.549	0.061 .061	1.63	1.600	0.06
Tittabulg	10L 28R	GAC	1.5	1.43 1.49	1.549 1.626	.061	1.35	1.600	.06
Portland, Maine	11 29	AC	1.0	1.54 1.41	0.737 .762	0.029 .030	1.27	0.737	0.02
ortland, Maine	18 36	AC	1.0	1.86 1.83	.254	.010 .011	1.77	. 279	.01
Rochester, N.Y.	10 28	AC	1.0	1.74 2.18	0.559 .178	0.022	1.79	0.356	0.01
notice beer, not	4 22	PCC	1.0	3.68 4.50	.102	.004 .005	3.60	.152	.00

 a Number on right of column represents the runway transverse slope in percent.

^bUnder construction.

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Table 17. Concluded

Year	Airport	Runway	Year	Airport	Runway
1070	Hahn AB - M	11/29	1973	St. Louis Lambert - C	6/24
1970	RAF Milden Hall - M Wiesbaden AB - M	1/29 8/26		Aberdeen (S. Dak.) - C Farmington (N. Mex.) - C	13/31 7/25
1971	Dallas NAS - M Gallup (N. Mex.) - C	17/35 6/24		Greensboro-High Point - C Hill AFB - M	14/32 14/32
1972	Denver Stapleton - C Denver Stapleton - C Great Falls Int C Hot Springs (Va.) - C Nashville Metro C	8L/26R 8R/26L 16/34 6/24 2L/20R	1974	Las Vegas (Nev.) - C RAF Bentwaters - M RAF Lakenheath - M Roswell (N. Mex.) - C Sioux City (Idaho) - C	7/25 7/25 6/24 17/35 17/35
40	Sioux Falls (N. Dak.) - C Springfield (Mo.) - C Vernal (Utah) - C Wichita Mun C	15/33 13/31 16/34 N/A	1975	Boise (Idaho) - C Jackson Hole (Wyo.) - C Jamestown (N. Dak.) - C Las Vegas (Nev.) - C	10R/28 18/36 12/30 1R/19L
2	Bellingham (Wash.) - C Cedar City (Utah) - C Pease AFB - M Portland (Maine) - C	16/34 2/20 16/34 11/29		Missoula (Mont.) - C Monroe (La.) - C Pierre (S. Dak.) - C	11/29 4/22 13/31
1973	RAF Alconbury - M Rapid City (S. Dak.) - C Ramstein AB - M	12/30 14/32 9/27			
1	Salt Lake City (Utah) - C Salt Lake City (Utah) - C	16L/34R 16R/34L			

Table 18. U.S. porous asphalt runway surface construction