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PROFILE: Gradient Simulation for Rail Hump Classification Yards

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Designers of rail hump yards traditionally execute a long, tedious manual process to optimally design hump grades and retarder placements. This design process entails checking the velocities and headways of a worst-case sequence of cars to ensure that proper values of these variables can be maintained on the gradient. The computer simulation model PROFILE automatically computes these quantities and thus frees the designer from tedious work and allows him or her to generate and study more design alternatives. The model uses the usual static (velocity-independent) rolling-resistance formulation of car rollability but includes the option of using velocity-dependent rolling resistance. User input requirements and program-generated output are described, and an example of the application of the model to a typical design problem is given.

In rail hump yards, classification is performed by rolling a cut of cars down a grade and switching the cars into various classification tracks. To perform switching properly, sufficient headway between cars must be created and maintained. The principal problems in the design of the hump profile and in the development of an effective speed-control scheme are to ensure that (a) the headway maintained in the switching area [e.g., 15.2 m (50 ft)] is sufficient to throw switches and prevent catch-up in retarders, (b) speed restrictions [e.g., 24.1 km/h (15 miles/h)] at switches and curves are observed, and (c) proper coupling occurs on the class tracks within specified speed limits [e.g., 3.2-9.7 km/h (2-6 miles/h)]. Controlling headway and speeds would not be difficult if all cars had identical characteristics and rolling resistances (or rollability) because the initial time separation established at the crest would result in a uniform and predictable headway between cars.

However, car rollability is not uniform; it varies with weather and type of car and changes during the rolling of a car. Nonetheless, the profile designer must ensure that a large percentage of the cars (e.g.,

99.9 percent) are delivered to the bowl tracks in a manner that satisfies the above design constraints. Moreover, because car speed is directly translatable into hump throughput, it is desirable that the fastest car speeds meeting these constraints be used.

Achievement of those aims is usually approached by considering the hardest-rolling (slowest) and easiest-rolling (fastest) cars. Hump grades are usually designed to deliver the hardest-rolling car to the clear point at a specified speed [e.g., 6.4 km/h (4 miles/h)] or to a specified distance into the classification track [e.g., 152.4 m (500 ft)]. The sizing and placement of retarder sections are usually determined by examining a worst-case triplet of a design hardest-rolling car followed by a design easiest-rolling car followed by a design hardest-rolling car traveling to the last switch on the farthest outside track. The retarders are placed where the separation between the two lead cars becomes less than a specified value; the retarder slows down the second car to reestablish proper headway. The length (power) of the retarder is based on the amount of energy that must be removed from the second car in this worst-case situation (of course, railroad policies may require sufficient retarder power to stop any car). At the same time, caution must be exercised to ensure that the second (easiest-rolling) car is not slowed so much that the third (hardest-rolling) car catches it.

The purpose of the PROFILE model is to provide the yard designer with an iterative and interactive computer design tool to perform such an analysis and to ensure that the design constraints are satisfied. The need for some automation of the hump design procedure has long been recognized. The labor and hours involved in plotting velocity head diagrams and converting them to car velocity, integrating velocity of cars to obtain time-distance plots, and finally comparing time-

distance plots of cars to obtain headway have severely restricted the number of design alternatives that the yard designer could consider. The PROFILE simulation model is intended to automate this process, and the automation also offers the designer the option of selecting a more advanced model of car rollability (over the usual static-rolling-resistance formulation), if desired. PROFILE does not automate the entire yard design process or replace the designer; it extends the abilities of the designer by permitting him or her to evaluate many more design alternatives in shorter time than is possible in the manual process.

The PROFILE model has been used to support the yard design efforts of the Boston and Maine Corporation (1), the Consolidated Rail Corporation (2), and the Union Pacific Railroad (3).

OVERVIEW OF THE MODEL

PROFILE is a one-track simulation; that is, the user selects one route from the crest to the bowl and simulates only that route in a run. With repeated runs, all routes to the bowl can be simulated, if necessary. The profile gradient along this route is represented as a series of track sections. All parameters are assumed to be constant within a given track section.

Only single-car cuts are modeled, although longer cuts can be approximated as a single car of unusual length.

Within each track section, each car is treated for the purpose of its dynamics as a point mass, the motion of which is assumed to be governed by the following differential equation:

$$d^2X/dt^2 = dV/dt = \alpha + \beta V \tag{1}$$

$$\alpha = g_e [\tan \theta - \mu - C - W - (S/L) - (R/L)] \tag{1a}$$

$$\beta = g_e (-\mu_v - W_v) \tag{1b}$$

$$g_e = [T/(T + I)] g \tag{1c}$$

where

- X = distance from an arbitrary origin (m);
- t = time (s);
- V = velocity of the car (m/s);
- α = sum of all static terms that contribute to the car's acceleration (m/s²);
- β = sum of all velocity-dependent terms that contribute to the car's acceleration (s⁻¹);
- g_e = effective acceleration of gravity used to account for energy stored in the rotating wheels of the car (m/s²);
- g = acceleration of gravity (m/s²);
- θ = angle of the grade below horizontal;
- tan θ = grade (downgrades taken positive) (m/m);
- μ = static rolling resistance (N/N);
- C = curve resistance, if the track section is on a curve (N/N);
- W = wind resistance (N/N);
- S = velocity head lost in switch, if the track section is a switch (m);
- L = length of track section (m);
- R = velocity head extracted by retarder (if the track section is a retarder) (m);
- μ_v = velocity-dependent resistance coefficient (N/N per m/s);
- W_v = velocity-dependent wind resistance coefficient (N/N per m/s);
- T = weight of the car (kg); and
- I = additional weight of the car to account for the

rotation of the wheels (kg).

Obviously, in any given track section, not all the terms will be applicable. For example, a conventional retarder and a switch would never be found in the same track section. The various parameters are assumed to be constant within each track section; whenever any parameters change, a new track section must be specified. This happens, for example, in specifying the beginning and end of a retarder. Specification of a new track section is also required whenever the grade changes. Vertical curves are approximated by a series of track sections of constant grade.

The solutions of the differential equations for β ≠ 0 and taking V = V₀ and X = X₀ at t = 0 are

$$V = (\alpha/\beta) + [(\alpha/\beta) + V_0] \exp(\beta t) \tag{2}$$

and

$$X = X_0 - (\alpha/\beta)t - (1/\beta) [(\alpha/\beta) + V_0] [1 - \exp(\beta t)] \tag{3}$$

For β = 0 (i.e., only static rolling resistance), the solutions reduce to the well-known case of uniformly accelerated motion (for the above boundary conditions), as follows:

$$V = V_0 + \alpha t \tag{4}$$

and

$$X = X_0 + V_0 t + (1/2) \alpha t^2 \tag{5}$$

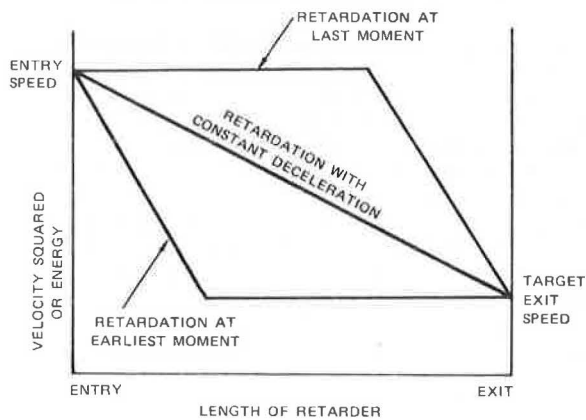
The β = 0 case is the usual static-rolling-resistance formulation, for which computational techniques based on energy relations are well developed. However, although these energy relations are easily applied to obtain velocity, integrating the velocities over a varying grade to obtain distance-time plots and hence headways between cars can become tedious. Even when a static-rolling-resistance formulation is being used, PROFILE has great utility as a quick means of calculation.

Although wind resistance would usually be handled by a V² term, in PROFILE only a V term is used. At the low speeds in a hump yard, the curvature of a V² relation should be sufficiently slight that it can be satisfactorily approximated by a linear term.

The retarders treated in the present version of PROFILE are the conventional clasp type, which are usually controlled by a process control computer. Currently, PROFILE does not consider the distributed types of retarders that offer quasicontinuous control through purely mechanical-hydraulic analog logic systems (as offered by certain European vendors). The conventional retarder system is quite complex: The process control computer controls both the overall amount of retardation and the detailed dynamics of car-retarder interactions while the car is within the retarder. Several algorithms are in use to decelerate the car within the retarder. They are all based on achieving a desired exit speed from the retarder. These algorithms can be roughly categorized into three types, as shown in Figure 1 and discussed below:

1. Retardation at the earliest moment--Retardation at the earliest moment is probably the most common algorithm for retarder control (4-6). The retarder closes as soon as the car enters; when the car reaches the exit velocity, either the retarder opens and the car rolls freely for the rest of the length of the retarder or the retarder opens and closes in an attempt to maintain

Figure 1. Retarder deceleration algorithms.



the car at approximately the desired exit speed. This scheme tends to restrict hump throughput because the car travels at minimum average speed for the length of the retarder. It also causes a disproportionate amount of retarder wear to occur near the front.

2. Retardation at the last moment (4)—The algorithm for last-moment retardation is based on a prediction of the rollability of the car and the power of the retarder. The retarder initially remains open when the car enters it. By using the predicted parameters, the retarder is then closed just at the time expected to produce deceleration of the car to the desired exit velocity. This scheme generally permits a high throughput because the car moves at maximum speed throughout the retarder. This algorithm, however, lacks a safety margin for cases in which the car rolls faster than predicted because of errors in predicting rollability, grease on the wheels or rails, or the like. This algorithm also causes a disproportionate amount of retarder wear to occur near the rear of the retarder.

3. Retardation with constant deceleration—Under the constant-deceleration algorithm, the retarder is commanded either to open and close several times (7) or to exert a constant retardation force (5); in either case, the aim is to achieve the desired exit speed with approximately constant deceleration. Some modern commercial retarder systems achieve this ideal at least approximately (8). This scheme maintains better throughput than algorithm 1 and maintains a safety reserve of retarder power that is lacking in algorithm 2. It also causes the retarder to wear approximately uniformly throughout.

In the PROFILE model, the third type of deceleration scheme, constant deceleration, is assumed to apply. Under constant deceleration, energy (i.e., velocity head) is extracted at a uniform rate during the car's transit of the retarder, and the total amount of velocity head extracted within the retarder, when divided by the retarder length, acts simply as an additional resistance term—hence, its appearance in Equation 2.

PROGRAM DESCRIPTION

PROFILE is a time-step simulation written in ANSI standard FORTRAN. Events are assumed to occur either at integral multiples of a predetermined time step Δt or within the time step for certain easily calculated events (such as the entry of a car into a new track section). The time-step method has been selected because of the ease it affords in the calcula-

tion of transcendental solutions to differential equations.

The simulation starts by humping the first car at simulation clock time zero. From the length of the cars involved and the hump speed, the hump time for the second car is computed and stored until the simulation clock is equal to that hump time. At the calculated hump time, the second car is humped and put into the system. The hump time for each car is so computed until all cars that the user wishes to put into the system are humped.

Once a car has been humped, movement of cars along the track is accomplished by advancing the simulation clock in increments of Δt . At each time step, the differential Equation 1 is solved for the instantaneous velocities and the distances of cars along the track. Each time a car enters a new track section, the program solves an initial-value problem based on the general solution to the differential equation and the specified configurations of the new track segment. These coefficients are used in subsequent calculations for this car on the track at steps of Δt until the car leaves the track section.

At each time step, the coupler-to-coupler headways between the cars in the system are checked to maintain a safe operating distance between the cars and to avoid mis-switching, catch-up in retarders, and collisions. If headway is insufficient, the program writes a warning message to the output file. If a collision occurs or if a car stalls, the program stops and writes a message to the output file. These messages show the simulation clock time when the catch-up occurred, the distance along the track for each car, and the velocities of the cars at that time. The user can then analyze the output and change retarder placements, the length of the retarder, or any other parameter and start a new computer iteration.

Data on each car are collected at each print interval as specified by the user. For each car, the simulation clock time, instantaneous velocity, velocity head, distance from the hump crest, and distance and time headways from the preceding car are written to and stored in a print buffer. Data in the buffer are written to the output file whenever the simulation stops. If no collision or stall occurs, the simulation stops when the last car has come to the end of the last track section.

Figures 2-4 show sample partial outputs (the program is calibrated in U.S. customary units of measurement).

DESCRIPTION OF INPUT

The first input variables are general: the time step Δt , the hump speed, the data print interval, switches controlling the printing of tables and plots, and the printer width (in characters). To model the occurrence of events accurately, the time step chosen should be sufficiently small but not so small as to cause an inordinate increase in running time (1 s is usually satisfactory). Data output frequency is controlled by the data print interval variable, which should be chosen in integral multiples of the time step but should never be less than the time step.

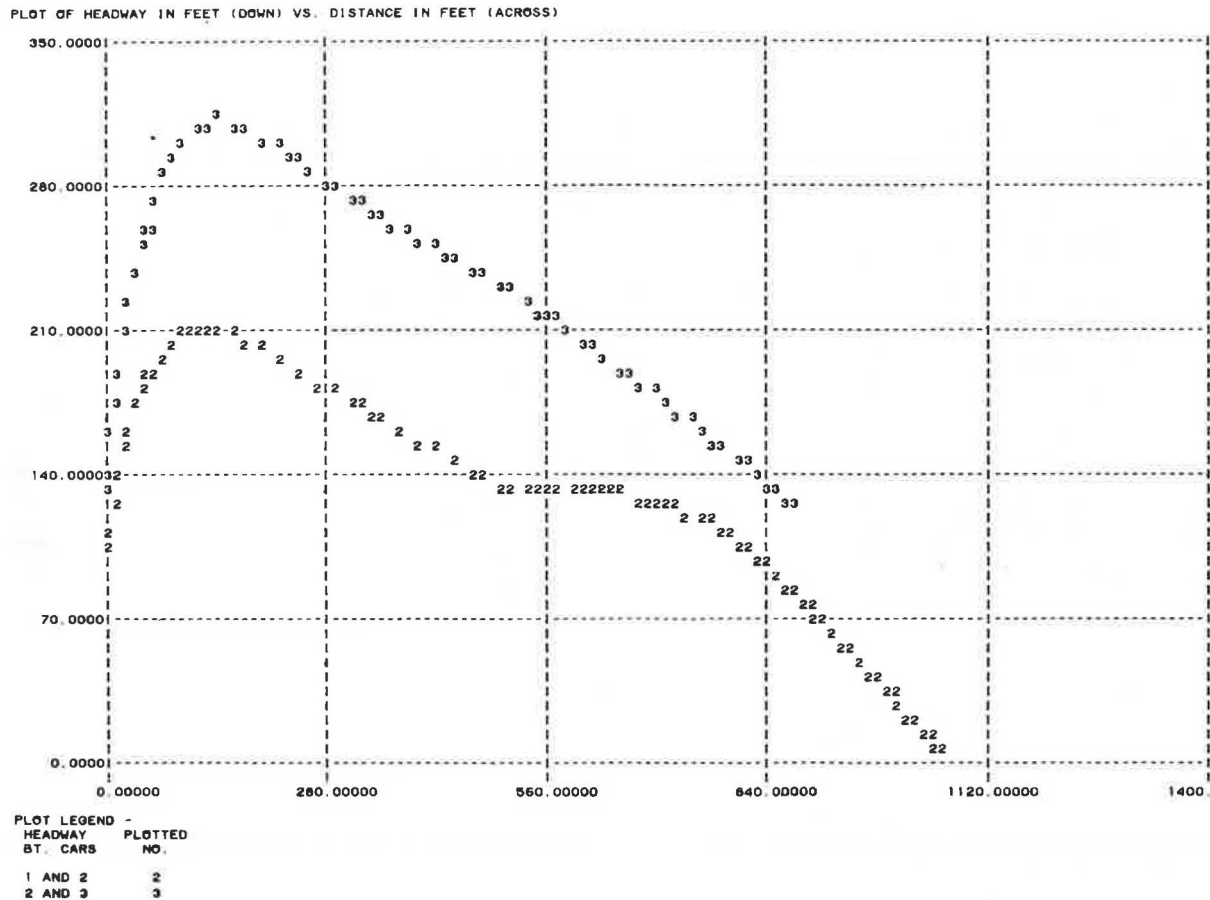
Next, the following data for the track sections are specified (in U.S. customary units):

1. Length of track section (ft);
2. Grade of track (percent);
3. Rolling resistance, static, easy roller (lbf/ton-f);
4. Rolling resistance, static, hard roller (lbf/ton-f);

Figure 3. Example of car history table: partial output of car 2 (easy roller) for trial run 2 of Yermo Yard.

CAR NO.	2									
CAR TRAVEL TIME (SEC)	SYSTEM TIME (SEC)	DISTANCE ALONG TRACK (FT)	DISTANCE HEADWAY BETWEEN PREC CAR (FT)	TIME HEADWAY BETWEEN PREC CAR (SEC)	INSTANTANEOUS VELOCITY (FT/SEC)	INSTANTANEOUS VELOCITY (MPH)	VELOCITY HEAD (FT)	TRACK SECTION NUMBER	TRACK SECTION DESCRIPTION	
0.000	16.364	0.000	102.770	7.382	3.667	2.500	.210	0/ 1	****TRACK SECTION BOUNDARY****	
.636	17.000	2.515	111.691	7.772	4.236	2.888	.281	1	CREST TO EVC	
1.636	18.000	7.198	125.778	8.328	5.131	3.499	.412	1	CREST TO EVC	
2.636	19.000	12.777	139.547	8.823	6.026	4.109	.568	1	CREST TO EVC	
3.636	20.000	19.290	152.447	9.266	6.921	4.719	.749	1	CREST TO EVC	
4.636	21.000	26.619	164.281	9.663	7.816	5.329	.956	1	CREST TO EVC	
5.636	22.000	34.883	174.850	10.022	8.711	5.939	1.187	1	CREST TO EVC	
6.636	23.000	44.041	184.238	10.347	9.606	6.550	1.443	1	CREST TO EVC	
7.240	23.603	50.000	189.322	10.830	10.146	6.918	1.610	1/ 2	****TRACK SECTION BOUNDARY****	
7.636	24.000	54.126	192.394	10.642	10.657	7.266	1.777	2	EVC TO FORMER M. RET.	
8.636	25.000	65.427	199.029	10.892	11.945	8.145	2.232	2	EVC TO FORMER M. RET.	
9.636	26.000	78.016	204.087	11.097	13.234	9.023	2.740	2	EVC TO FORMER M. RET.	
10.636	27.000	91.894	207.583	11.263	14.522	9.901	3.299	2	EVC TO FORMER M. RET.	
11.636	28.000	107.060	209.683	11.395	15.810	10.779	3.910	2	EVC TO FORMER M. RET.	
12.489	28.852	121.000	210.356	11.484	16.908	11.528	4.472	2/ 3	****TRACK SECTION BOUNDARY****	
12.636	29.000	123.513	210.389	11.499	17.087	11.651	4.567	3	FORMER MASTER RET.	
13.636	30.000	141.206	209.688	11.574	18.299	12.477	5.238	3	FORMER MASTER RET.	
14.636	31.000	160.111	207.669	11.599	19.510	13.302	5.953	3	FORMER MASTER RET.	
15.636	32.000	180.227	204.324	11.557	20.722	14.128	6.717	3	FORMER MASTER RET.	
16.242	32.806	193.000	201.725	11.494	21.456	14.629	7.201	3/ 4	****TRACK SECTION BOUNDARY****	
16.636	33.000	201.491	199.877	11.439	21.612	14.735	7.306	4	FORMER M. RET. TO KING SW.	
17.349	33.713	217.000	196.445	11.323	21.895	14.928	7.499	4/ 5	****TRACK SECTION BOUNDARY****	
17.393	33.759	218.000	196.189	11.315	21.825	14.881	7.451	5/ 6	****TRACK SECTION BOUNDARY****	
17.636	34.000	223.289	195.010	11.272	21.848	14.896	7.467	6	KING SW TO LAP	
18.538	34.901	243.000	190.603	11.096	21.935	14.955	7.526	6/ 7	****TRACK SECTION BOUNDARY****	
18.583	34.847	244.000	190.381	11.086	21.881	14.899	7.469	7/ 8	****TRACK SECTION BOUNDARY****	
18.636	35.000	245.158	190.127	11.075	21.847	14.896	7.466	8	LAP SW TO PT	
19.636	36.000	266.962	185.424	10.859	21.762	14.838	7.408	8	LAP SW TO PT	
20.636	37.000	288.682	180.901	10.622	21.677	14.780	7.351	8	LAP SW TO PT	
21.636	38.000	310.317	176.535	10.373	21.593	14.722	7.293	8	LAP SW TO PT	
22.636	39.000	331.868	172.104	10.118	21.508	14.668	7.236	8	LAP SW TO PT	
23.201	39.565	344.000	169.520	9.971	21.460	14.632	7.204	8/ 9	****TRACK SECTION BOUNDARY****	
23.636	40.000	353.351	167.483	9.855	21.502	14.660	7.232	9	PT TO GR. RET.	
24.636	41.000	374.900	162.885	9.576	21.598	14.726	7.297	9	PT TO GR. RET.	
25.636	42.000	396.546	157.381	9.286	21.694	14.791	7.362	9	PT TO GR. RET.	
26.636	43.000	418.288	151.879	8.989	21.790	14.857	7.427	9	PT TO GR. RET.	
27.585	43.949	439.000	146.418	8.731	21.881	14.919	7.489	9/10	****TRACK SECTION BOUNDARY****	
27.636	44.000	440.124	146.116	8.717	21.811	14.871	7.442	10	GR. RET.	
28.636	45.000	461.287	140.879	8.484	20.456	13.947	6.546	10	GR. RET.	
29.636	46.000	481.035	136.734	8.337	19.100	13.023	5.707	10	GR. RET.	
30.636	47.000	499.458	133.733	8.266	17.745	12.099	4.926	10	GR. RET.	
31.636	48.000	516.525	131.878	8.257	16.390	11.175	4.202	10	GR. RET.	
32.636	49.000	532.237	131.168	8.317	15.035	10.251	3.536	10	GR. RET.	
33.096	49.459	539.000	131.229	8.368	14.412	9.827	3.249	10/11	****TRACK SECTION BOUNDARY****	
33.636	50.000	546.793	131.433	8.435	14.412	9.827	3.249	11	GR TO LAP 2	
34.273	50.639	556.000	131.627	8.612	14.412	9.827	3.249	11/12	****TRACK SECTION BOUNDARY****	
34.345	50.709	557.000	131.649	8.520	14.279	9.735	3.189	12/13	****TRACK SECTION BOUNDARY****	
34.636	51.000	561.158	131.760	8.556	14.255	9.719	3.179	13	LAP 2 TO HF 2	

Figure 4. Distance headway versus distance for trial run 2 of Yermo Yard.



distance, (b) speeds of all cars versus distance, and (c) distance headways between all cars versus distance (Figure 4).

APPLICATION OF THE MODEL

The sample application problem described in this section is based on a modified specification for the Union Pacific Railroad's Yermo Yard in southern California. The hump profile design requires several levels of decision making on cost- and performance-related matters. The considerations on cost and performance would be reflected in the retarder types to be used, hump-crest height, humping speed, impact speed, and number of mis-switched cars. After having determined the type of retarder and retarder configuration to be adopted, the designer must iteratively examine both the horizontal and vertical design to arrive at a final design that satisfies the specified goal.

The application problem discussed here is only one stage of the process of hump profile design in which a given profile design is evaluated and modified to a better design through iterations of PROFILE runs.

The design, as used in trial run 1 (not shown in the figures) in this example, has a master retarder of 28.3 m (93 ft) and three group retarders of 30.5 m (100 ft). Each group retarder leads to 10 classification tracks. The distance between the hump crest and the tangent point of the outermost track is 323.4 m (1061 ft).

The runs for this design were based on the simulation of a conventional hard/easy/hard-rolling triplet of cars. A worst-case condition was assumed: Since the easy-rolling car is going to a nearly full-class track, it must be retarded to a low target speed by the tangent point [9.6 km/h (6 miles/h)]; meanwhile, since the hard-rolling car must penetrate as far as possible an adjacent empty-class track, its retardation is minimal.

The objective of the study was to test the feasibility of the design by examining the following design requirements:

1. The hump speed is at least 4.0 km/h (2.5 miles/h), 3.67 cars/min.
2. The hard roller must not stall before the tangent point.
3. The maximum speed of the easy roller at the tangent point is 9.6 km/h (6 miles/h).
4. The maximum speed of a car in the switch segments is 24 km/h (15 miles/h).
5. The coupler-to-coupler headway is at least 15.2 m (50 ft) at each switch.
6. There is never more than one car in the same retarder at any time.
7. No catch-ups should occur before the clearance point of each track.

The major assumptions used in the design process were the following:

1. Only static rolling resistances apply.
2. The hard roller has a rolling resistance of 9 N/kN (18 lbf/ton·f) between the hump crest and the exit from the group retarders and 9 N/kN (10 lbf/ton·f) thereafter.
3. The easy roller has a rolling resistance of 2 N/kN (4 lbf/ton·f) between the hump crest and the exit from the group retarder and 1 N/kN (2 lbf/ton·f) thereafter.
4. The velocity head loss attributable to each switch is 0.018 m (0.06 ft) when the car travels along the curved track and is assumed to be zero if a car travels on the straight track. This value is constant for all

turnout numbers.

5. The velocity head loss attributable to a curved section of track is 0.012 m (0.04 ft) per degree of deflection angle.

6. The average car length is 18.3 m (60 ft).

7. The average car weight is 58 Mg (64 tons) for the hard roller and 122 Mg (135 tons) for the easy roller.

8. The extra weight of the car attributable to wheel rotation is 0.91 Mg (1 ton).

9. The wind resistance is zero.

A general interactive and iterative design procedure was used here to select an example design. The steps in this procedure are the following:

1. Select the configuration and type of retarder and the method of retardation.
2. Determine the car-speed constraints at the tangent point and at other points along the track.
3. Design a trial horizontal layout.
4. Determine the hump height from steps 2 and 3.
5. Select the trial grades along the track.
6. Run PROFILE.
7. Examine the output. If the result is satisfactory, go to step 8. If the result shows speed violations, go back to step 3. If the result contains catch-up problems, go first to step 5. If the catch-up problem cannot be solved by changing grades, go to step 3.
8. Determine whether any segment, especially the retarder segment, is excessively long; if so, go to step 3. Otherwise, the design is complete.

It should be noted that other procedures not shown here have been developed to enable the PROFILE user to select a hump speed and a retarder control policy.

The example discussed here illustrates one step of the interactive and iterative design procedure presented above. The objective in trial run 2, the partial output of which is shown in Figures 2-4, was to try to eliminate the master retarder. This change necessitated shortening the distance between the hump crest and the first switch by 6.4 m (21 ft), which shortened the distance to the tangent point to 317.0 m (1040 ft). A comparison between the collision-related output for trial run 1 (not shown) and the same information for trial run 2 (Figure 2) revealed that the collision point decreased from 398.1 to 334.0 m (1306-1099 ft) from the hump crest. Since the latter value is still well past the clearance point (in fact, past the tangent point), the design of trial run 2 satisfies the design requirements. Examination of other performance measures output by the model, as shown partially in Figures 2-4, reveals that all other design requirements are also met by the design of trial run 2. Under the assumptions used in this example, the design changes effected between trial runs 1 and 2 demonstrate a considerable cost reduction and point up the advantage of having the PROFILE model available to try such "what if" experiments.

Figure 3 shows a part of the output for car 2 (the easy roller). All of the necessary data related to the movements of car 2 are included in this table.

From the plot of speeds of the cars as a function of distance (not shown) or data such as those in Figure 3, it has been determined that the easy roller in trial run 2 attains a maximum speed of slightly less than 24 km/h (15 miles/h). This satisfies the maximum-speed constraint in the switching area. It can also be verified that the easy roller satisfies the 9.6-km/h (6-mile/h) speed constraint at the tangent point and that the unretarded hard roller satisfies both speed constraints.

Figure 4 shows a plot of distance headway between

successive cars. The number 2 indicates the headway between cars 1 and 2, and 3 indicates the headway between cars 2 and 3. Figures 3 and 4 show that sufficient headway exists between cars to detect individual cars and to throw the switch in all switch segments.

FURTHER WORK AND CONCLUSIONS

Further work is in progress to enhance the interactive capability of the PROFILE program. Specifically, simplifying the user input procedures and increasing the amount of graphical output are being considered. In addition, more work is required to characterize and quantify the nature of car rollability. Freight-car rolling behavior, which is essentially an input to PROFILE, is a critical determinant of the final profile design.

This paper has shown that PROFILE can be used to eliminate the tedious manual process of evaluating hump profile designs by using scale drawings. In addition, PROFILE gives a precise prediction of catch-up problems between cars. The program allows the yard designer to evaluate many more design alternatives than it was previously possible to evaluate, thus ensuring production of the most cost-effective design.

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Conflicts Between Urban Areas and Railroads: A Status Report

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The development of conflicts between urban areas and railroads in the United States is examined, and the nature and magnitude of the current problems and present and past efforts to resolve them are described. Many American cities developed primarily as a result of the railroads, but changes in urban activities and transportation operations have altered somewhat the relation between the cities and railroads. Continuing expansion of urbanized areas and increases in vehicle travel have intensified the conflict. Cities have reacted by pushing for elimination of railroad-highway grade crossings and, in some cases, for consolidation, relocation, and/or removal of railroad tracks from the center city. Many city planners see the railroads as a hindrance to rejuvenation efforts. In some cities, underutilized railroad properties are in strategic locations that could be important in urban redevelopment plans. High-volume rail lines that pass through congested downtown areas can cause massive traffic jams and delays unless crossings are grade separated. Railroad-highway grade crossings pose safety problems to the motorist and restrict mobility, which is particularly important for emergency vehicles. In addition, the slow train speeds mandated by

local municipalities, frequent grade crossings, and large numbers of trespassers are not compatible with efficient railroad operation. But new rail routes are difficult to locate and expensive to build, and there are many implementation problems involved in other, less expensive solutions, such as consolidation or abandonment.

Conflicts between U.S. railroads and urban communities have existed, to varying degrees, ever since railroad operations began in 1830. Initially, most of the concerns about urban railroads had to do with safety. Safety problems included dangers associated with grade crossings, runaway trains, and derailments. However, since train speeds through towns were relatively slow and vehicle traffic crossing tracks was of low volume, the safety of rail operations in urban areas