Moisture & Compaction Measurement during Unbound Aggregate Layer Construction

Hosted by Andrew Dawson University of Nottingham, UK



chair, TRB Committee AFP70 "Aggregates"

Your presenters

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- Office of Road Research, MnDOT

• Erol Tutumluer

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Unsaturated Geomechanical First Principles

Claudia Zapata Arizona State University



Sources of Moisture in Pavements



Key Findings Related to Moisture Effects on Resilient Modulus

Field evidence and numerous numerical modeling studies have shown that even though the pavement structure acts as a cover for the unbound material...

... its moisture content eventually reaches an equilibrium condition, fact that it is greatly influenced by climate and soil properties.

Controlling Factors



- Unbound material reaches an equilibrium condition
 - Climate
 - Soil properties
- Microclimate controls flux boundary conditions

Unsaturated Soils

- One-third of earth's surface is considered arid or semi arid
- Unbound materials under pavements are generally unsaturated



- After decades of focus on saturated soils, the Geotechnical profession has begun to turn its attention to unsaturated soils.
- Construction in unsaturated soils is preferred when practical, due to reduced costs and effort.
 - Research community has made substantial advances in understanding fundamental aspects of unsaturated soil behavior.

Typical Pore Water Pressure Profile

- Matric suction controls the water content in the soil
- If we know the suction at equilibrium, we can approximate the water content at which the soil will try to equilibrate



Some Case Study & Relationship to Pavement Design

John Siekmeier Office of Materials & Road Research MnDOT



Acknowledgements

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- Other DOTs and Federal Agencies
- Private Sector Consultants
- Product Manufacturers and Contractors
- Universities

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Current Situation

- Pavement foundations are important and are constructed to avoid saturation where possible.
- MnPAVE is MnDOT's mechanistic pavement design method used to quantify performance.
- Unsaturated geomaterials have greater moduli, which can be used to optimize pavement designs.

Project Objective

• Pavement design procedures are modified to better utilize unsaturated geomechanics so that we build more financially effective roadways.

Pavement Foundations are Important



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Minnesota Department of Transportation

Office of Materials & Road Research 1400 Gervais Avenue, MS 645 Maplewood, MN 55109

Memo

- TO: PCMG, CMG, MnDOT Districts, Materials Engineers, Soils Engineers, State Aid
- FROM: Glenn M. Engstrom, Director Office of Materials & Road Research
- DATE: October 31, 2014
- SUBJECT: Pavement Design Manual Publication

I am pleased to announce the publication of the MnDOT Pavement Design Manual.

This publication represents a significant effort to update pavement design procedures and codify existing documents into a single point of reference. As of November 1, 2014, all MnDOT pavement designs shall follow the pavement design, pavement-type selection, LCCA, and alternate bidding as laid out in the Pavement Design Manual. To view the manual, please follow http://www.dot.state.mn.us/materials/pvmtdesign/newmanual.html

Mechanistic Pavement Design

- Provides the framework for using performance based material properties
- Free pavement design software available <u>http://www.dot.state.mn.us/app/mnpave/index.html</u>
- Just Google "MnPAVE"

Need Mechanistic Design Inputs

🕀 MnPAVE - MnPAVE1	
File Edit View Window Help	
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Structure Confidence Level (50-99) 70 Use Mean Values Overburden Calculation View Thickness Values Coefficient of Variation Adjusted Thickness Edit Structure Thickness Layers Material 1 HMA 2 AggBase 3 Subbase 12 4 4 EngSoil 5 UndSoil	Basic Intermediate Advanced Design Mode Use values from Basic Design Level Use values from Intermediate Design Level Advanced mode (enter values now) Parameter Shown Below Parameter Shown Below Structural Number = 3.7 Design Modulus, ksi Adjusted Poisson's Ratio Structural Number = 3.7 Seasonal Modulus Multipliers Calculate HMA Modulus Weekly Early Late Fall Winter Spring Spring Spring Spring 1 10 0.36 0.7 0.85 1 10 10 0.7 0.85
Units	Simulate FWD Simulate LWD View Damage Equations
C SI Control Panel	View Pavement Input Moisture Temperature Equation Characteristics

Need Seasonal Change in Aggregate Water Content



MnROAD Test Section Case Studies



Lessons Learned from Case Studies

- Modulus greatly affected by moisture suction (tensile stress between aggregate particles)
- Suction tensile stress depends on:
 - Quantity of sand, silt, and clay particles
 - Distribution of particles and voids
 - Particle shape and void shape
 - Porosity (measure of void space)
 - Moisture content (measure of water in voids)



Lab Resilient Modulus

Numerical Simulation

Parameters Studied

- Aggregate gradation
- Moisture content (suction/tension)
- Friction between particles (roughness)
- Confining stress

Increasing confining stress increases resilient modulus.

Increasing suction increases resilient modulus.

Aggregate Base Unsaturated Gain Factors

particle friction 0.6 and contact bridge 1 mm (less sand and smaller particles)

Suction	Gain Factor	Gain Factor
(kPa)	@ 50 kPa and 0.05% strain	@ 100 kPa and 0.05% strain
5	1 32	1 49
0	1.02	1.40
30	1.90	2.07
60	2.26	2.36

Gain factors are the ratio of modulus at listed suction compared to 1 kPa suction.

Aggregate Base Unsaturated Gain Factors

particle friction 0.6 and contact bridge 10 mm (more sand and smaller particles)

Suction	Gain Factor	Gain Factor
(kPa)	@ 50 kPa and 0.05% strain	@ 100 kPa and 0.05% strain
Б	1 46	1 20
5	1.40	1.30
30	2.26	1.85
60	2.65	2.14

Gain factors are the ratio of modulus at listed suction compared to 1 kPa suction.

Damage vs Unsat Gain Factor

Damage must be less than of 1.0 to achieve 20 year design life.

Conclusions

- Modulus increases as suction increases and aggregate roughness increases.
- Pavement structures can be optimized by applying unsaturated geomechanics.
- Financial effectiveness can be enhanced by better utilization of limited resources.

Suction and Moisture Collected at Field Sites and Variation Information

Claudia Zapata Arizona State University

Moisture Data Collected at FAA Test Facility

CC7 Pavement Cross Section - South

Section View of the NAPTF Test Section
Moisture Sensors Installation

- Total of 30 sensors were installed throughout the test section
- Sensors have been collecting moisture every hour since July 2013



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11.01.0

Location E

Depth= 35 in = 0.89

Location F

Depth= 35 in = 0.89 m

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If measuring suction in the field is not an option...

 Equilibrium suction in the field can be predicted based on climatic indexes (such as the Thornthwaite Moisture Index) and readily available soil index properties

TMI-P₂₀₀ Model – Granular Bases



Perera, 2003

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Soil-Water Characteristic Curve Prediction

- Once the suction at equilibrium is predicted, the soil-water characteristic curve can be use to estimate the water content at equilibrium
- The SWCC can be
 - obtained in the laboratory
 - roughly estimated from grain-size distribution parameters and other soil properties

Estimating SWCC based on Grain Size Distribution



Estimating SWCC for Granular Material (Torres and Zapata, 2011)

SWCC Parameter of kPa	$a_f = -967 .21 D_{10}^2 + 218 .37 D_{10} -$	Once the
		parameters
SWCC Parameter bf	$b = -10^{(-0.0075a_f^3 + 0.1133a_f^2 - 0.3577a_f + 0.3061)}$	are replaced in
Sweet and the of	$v_f - 10$	the Fredlund
SWCC Parameter cf	$c_f = 0.0058a_f^3 - 0.0933a_f^3 + 0.4069a_f + 0.348$	and Xing
SWCC Parameter hr, kPa	$h_r = 100$	equation, the
		family of
for the SWCC Equation:	$CF(\psi) = \frac{(\psi - 0.9128)}{0.017}$	SWCC curves
	Where:	can be
	θw = Volumetric Water Content θsat = Saturated Volumetric Water Content	
	ψ = Matric Suction, kPa	obtained.
	GI = Group Index	

SWCC for Granular Material based on D₁₀



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Suction Measurements Database

- Compiled from National Resources
 Conservation Service (NRCS) database
 - Initially intended for agricultural purposes
 - Key soil properties useful in highway/pavement engineering
 - Joint agreement with the then Bureau of Public Roads (BPR)



http://nchrp923b.lab.asu.edu/

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CHRP 9-23B	
Welcome to the Arizona State University Step 1 Select State Massachusetts Use the dropdown menus to find the milepost coordinates or, if you already know your coordinates, enter below, Leave blank to center on state. Latitude: Longitude: Latitude: Get Map Reset Step 2 Wait a minute for the layer to load click on the map to see each soil unit's Map Character (MapChar), Use the silder bar, to zoom in orout, orgab the map to pan.	<image/>
Step 3 Generate Soil Unit Report Mapchar: GX0 Get Report Enter a Map character (Mapchar) into the box to generate the soil unit report.	Hondo Process Brook Reservation 1 1 1 1 1 1 1 1 1 1 1 1 1

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Soil Properties Available Includes SWCC Parameters

- Grain size distribution
- Atterberg limits
- Saturated hydraulic conductivity
- AASHTO classification
- Water table depth
- Depth to bedrock
- Porosity
- Fredlund and Xing SWCC parameters

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C	http://nchrp	923b.lab.asu.edu/SoilQuery.php?mapCh		

Properties of Soil Unit GX0

Map Character	r Map Unit K	ey	Map Unit N	ame		Componer	nt Na	ame				
GX0	668403	Scituate	-Montauk-Ca	anton (s31	23)	Montauk						
AASHTO Classification	AASHTO Group Index	Top Depth (in)	Bottom Depth (in)	Thicknes (in)	s %	Compone	nent Water Table Depth D Annual Min (ft) Be		Dep Bedro	Depth to edrock (ft)		
A-4	0	0	2	2	11	11 N/A		N/A				
A-4	0	2	27.2	25.2	11	11 N/A		N/A				
A-2-4	0	27.2	72	44.9	11	11 N/A		N/A	N/A			
CBR from Inde Properties	x Resilient N Index Pro	lodulus fron perties (psi)	n Passing #4 (%)	4 Passing (%)	; #10)	Passing # (%)	g #40 Passing #200 5) (%)		00 Pass m	0 Passing 0.002 mm (%)		uid Limit (%)
46.3	29754		72.5	65		52.5		42.5 12		1		5
44.3	28928		80	75		62.5		47.5 12		17		5
61.6	35688		80	75		50		30 9.5		9.5		
Plasticity Inde (%)	x Saturated Water Co	Volumetric ntent (%)	Saturated H Conductivit	Hydraulic ty (ft/hr)	draulic Parameter (ft/hr) (psi)		Parameter bf		Parameter cf		Parameter hr (psi)	
2	37		0.27506		1.1231		1.0687		0.7417		3000.03	
2	36		0.27506	2.0		0655 1.0066		066	0.7998		3000.03	
1	24		0.02751		6.706		1.1	1.1387 0.		0.6662 30		1
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Laboratory Measurement of Suction & Relating it to Resilient Modulus

Claudia Zapata Arizona State University



How to Obtain Soil Suction?



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How to Obtain Matric Suction?

- Laboratory measurements
 - Pressure plates, pressure membranes
 - Filter paper method
- Field measurements
 - Thermal conductivity sensor
 - Tensiometers
 - Gypsum blocks

Measuring Matric Suction

- One way to measure matric suction is to directly measure or control -u_w and u_a.
- Because u_w is commonly highly negative, measuring or controlling u_w in the lab often requires increasing u_a to avoid cavitation of water in the measurement device.
- The axis-translation technique is a common method used for the direct measurement of soil matric suction.

Axis-Translation in the Lab

- This procedure changes the atmospheric pressure in the chamber to move the origin of reference for the pore-water pressure from the standard level to the final air pressure in the chamber.
- This is why the procedure is called "axistranslation."
- Cavitation is prevented because water pressure in the measuring system does not become highly negative.

Methods Available to Measure Matric Suction

- Direct methods:
 - o Tempe cells
 - o Pressure cells
 - o Tensiometers



Mounted and Unmounted Ceramic Disks



Measuring Matric Suction

- High air-entry ceramic disks, which are uniformly porous and separate air and water, are used.
- If the disk is saturated with water, air cannot pass through it since the air-water interface resists the flow of free air.



Mounted and Unmounted Ceramic Disks



Modified from Fredlund and Rahardjo, 1993

Air-Entry Value

Table 4.3 Properties of High Air-Entry Disks Manufactured by Soilmoisture Equipment

Corporation¹ (Manufacturer's Results)

Type of Disks	Approximate Pore Diameter (x 10 ⁻³ mm)	Measured Air- Entry Value, $(u_a - u_w)_d$, kPa
1/2 bar High flow	6.0	48-62
1 bar	2.1	138-207
1 bar High flow	2.5	131-193
2 bar	1.2	241-310
3 bar	0.8	317-483
5 bar	0.5	> 550
15 bar	0.16	> 1520

1 Soilmoisture Equipment Corporation, Santa Barbara, CA, USA

Relating Suction and Resilient Modulus

How can the resilient modulus be adjusted for environmental conditions?



Models by Andrei and Witczak (2003)



- M_R = Resilient Modulus at any degree of saturation (S)
- M_{Ropt} = Resilient modulus at optimum degree of saturation (S_{opt}) or initial compaction conditions
- F_{env} = Environmental adjustment factor
- a, b, k_m , β = Regression parameters

Models by Andrei and Witczak (2003)



Resilient Modulus at Optimum

$$M_{Ropt} = k_1 \cdot p_a \cdot \left(\frac{\theta}{p_a}\right)^{k_2} \cdot \left(\frac{\tau_{oct}}{p_a} + 1\right)^{k_3}$$

- Universal model
 - Implemented in the ME-PDG for "unfrozen" unbound materials
 - Stress dependent model

More data collected indicated...



F_{env} conservatively predicted

F_{env} for fine
 grained
 materials
 underestimated
 at dry conditions

Environmental Factor as a Function of Soil Type



Moisture and Compaction Measurement and Field Performance

Erol Tutumluer University of Illinois at Urbana-Champaign



Rutting is the Main Performance Indicator



Moisture Content and Aggregate Quality Influencing Permanent Deformation Behavior



Repeated Load Triaxial Test Results

(Tutumluer et al., 2009; R27-1 Project; ict.illinois.edu)

Effect of Shallow Ground Water Table on Permanent Deformation Accumulation



(Erlingsson and Ingason, 2004)

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Dolomite (14-in. thick) over CBR=3 Subgrade



0.0% arushed 11% passing No. 200 signs BI_0

ICT R27-124 Project; (Kazmee & Tutumluer, 2015)

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Sustainability in Aggregate Layer Construction



Pavement Forensics – PANDA & Geo-Endoscopy



Strength assessment using DCP and PANDA penetrometer



Detection of water table and layer interface



Depth of Water Table & Pavement Layer Strength


Minimize Rutting Due to Shearing in Base

Resist shear deformation

within the aggregate base

 Crushed, high quality angular stone for higher stability

– Proper compaction!!! (Density, Density, Density...)





ICT R27-81 Research Project (Mishra & Tutumluer, 2013; ict.illinois.edu)

NCHRP SYNTHESIS 445

Practices for Unbound Aggregate Pavement Layers



A Synthesis of Highway Practice

TRANSPORTATION RESEARCH BOARD OF THE NATIONAL ACADEMIES NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM

NCHRP Synthesis 445

(Tutumluer, 2013)

Download from the TRB Website:

http://onlinepubs.trb.org/onlinepubs/ nchrp/nchrp_syn_445.pdf



Best Practices: Aggregate Layers

• Equipment:

- Mixing by stationary plant e.g. pugmill / rotary mixer.
- Use mechanical spreaders to avoid segregation & achieve grade control.
- Suitable vibratory compaction equipment.

• Mixing and Transporting:

- Plant mix aggregates and water to OMC +1% / -2%
- Transport to site avoiding segregation and loss of moisture.

• Spreading:

- Place at correct moisture & thickness by mechanical spreader.
- When thickness >13 inches, consider 500-ft long test section to demonstrate adequate compaction without particle degradation.

Aggregate Placement & Compaction







Methods to Control the Moisture Content of Unbound Aggregate Base/Subbase Layers



(NCHRP Synthesis 445 - 2013)

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Laboratory & In Situ Compaction (soft subgrade)



In-Place Modulus Measurement of Constructed Aggregate Layers

 Density is not a required input for Mechanistic-Empirical pavement design methods

✓ Resilient modulus (M_R) is used as a key input

✓ Using M_R for construction quality control may facilitate linkage with design methods

Methods to Determine the Resilient Modulus of Unbound Aggregate Materials for Use in Granular Base and Subbase Layers



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Moisture (Suction) Effects on Aggregate Base Course Resilient Modulus (M_R)



Different Methods for In-Situ Modulus Measurement











Need to Know What We Are Measuring !!







Comparing Field Moduli with Lab M_R



Field moduli do not always relate to material quality as good as Lab M_R

Modulus-Based Compaction Control

NCHRP Synthesis 445 (2013)

 Combine the aspects of in-place modulus measurement and construction quality control

✓ Key issues to consider

✓ Measurement Depth

- ✓ Induced Stress State (In relation to Strength)
- ✓ Proper Algorithms for Layer Modulus Estimation

✓ Ideal approach (Do not rely on any one measurement!)

- ✓ Density: Target Value ± Tolerance
- ✓ Moisture Content: Target Value ± Tolerance
- ✓ Layer Modulus: Target Value ± Tolerance

Development of Modulus-Based Compaction Control Specifications

Requirements:

NCHRP Project 10-84

- Should be based on field measures of the stiffness or modulus and moisture content
- Should directly account for the seasonal variation of the modulus of the compacted unbound aggregate
- Should be founded on a comprehensive review of the current literature on the long-term behavior of various soils and unbound aggregates in terms of the principles of unsaturated soil mechanics

Indiana and Georgia implement modulus-based compaction control for demonstration projects only

Effect of Moisture Conditions in the Field



- Increase in Moisture → Decrease in Field Modulus
 No clear trend in effect of dry density
- No clear trend in effect of dry density

Moisture (Suction) Effects on Field Modulus Increase



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Variation of response as detected by sensors in a roller compactor



Webinar Conclusions

- Moisture changes suction
- Suction changes resilient modulus & rutting resistance
 - Therefore granular layers in pavements are sensitive to moisture and other factors that change suction
- Tools available to measure suction and suction effects
- Better compaction occurs when suction low (near OMC)
- Density has value in providing basic mineral skeleton, but modulus is fundamentally important and is sensitive to suction/moisture effects.
- Assessing in-situ density or modulus by indirect methods must acknowledge moisture/suction effects

Question Time