

Slumps other than those shown above may be used, when in the opinion of the Engineer, conditions are such that satisfactory workability can not be obtained within limits specified

MIXED AGGREGATE

Mixed aggregate is defined as that material where fine sand makes up more than 50 per cent of total aggregate, with specification as follows

| | |
|-------------------------------------|------|
| Passing 2 inch mesh sieve | 100% |
| Passing 4 mesh sieve not less than | 60% |
| Passing 30 mesh sieve not less than | 15% |
| Gradation factor, not less than | 3 75 |

Concrete made with aggregates specified above, will be referred to as mixed aggregate concrete

COARSE AGGREGATE

| | |
|--|---------|
| Passing $1\frac{1}{2}$ inch mesh sieve | 95-100% |
| Passing $\frac{3}{4}$ inch mesh sieve | 35- 65% |
| Passing $\frac{3}{8}$ inch mesh sieve | 10- 30% |
| Passing 4 mesh sieve, not more than | 5% |
| Passing 2 inch mesh sieve | 95-100% |
| Passing 2 inch mesh sieve | 35- 65% |
| Passing $\frac{3}{8}$ inch mesh sieve | 10- 30% |
| Passing 4 mesh sieve, not more than | 5% |

Fine aggregate was either sand or flint sand. A combination of fine sand with correct amounts of either coarse aggregate listed above will be referred to as coarse aggregate concrete

CONCRETE STRUCTURES

Aggregates for structures were similar to those specified for concrete pavement and reference to them will be made under the same headings, that is, as mixed aggregate concrete and coarse aggregate concrete. The amount of water and minimum and maximum cement content

for class A structure concrete is listed below:

| Type of Aggregate | Bbls Mins | Cement per Cu Yd Max | Gals water per Sack of Cement |
|--|-----------|----------------------|-------------------------------|
| Crushed stone or gravel and fine aggregate | 1 25 | 1 65 | 6 25 |
| Mixed aggregate | 1 35 | 1 90 | 6 25 |

Approximate slumps used without vibrator

| | Inches |
|---------------------------|-----------|
| Mixed aggregate | 2 00-4 00 |
| Coarse aggregate concrete | 3 00-5 5 |

VIBRATORS FOR STRUCTURES

Generally, vibrators used for structures have been of two types. One is electrically driven, with a gas engine of two or two and one-half horsepower generating electricity which operates a motor installed in the vibrator. Tubing is used to carry the wiring and to raise and lower the vibrator in the forms. The vibrator on this type of machine is rather large and for that reason is better adapted to work where the reinforcing bars are spaced far enough apart, or far enough from the forms, so as not to handicap its use. This machine has a large advantage because its power supply can be stationary and the vibrator moved about over a considerable amount of work. The frequency range is between 1800 to 3500 V P M, and it is found to operate very satisfactorily on 2800 V P M.

Another type of internal vibrator used quite extensively has power furnished by a gasoline air cooled motor, to which is attached flexible tubing at the end of which is the vibrator. The revolutions are about the same on both types.

Experiments with the two types of vibrators were run on structures of mixed aggregate concrete in which slumps varied from 1 0 to 3 0 in. The vibrators

were operated at various frequencies and were permitted to agitate the concrete from five to thirty seconds. The surfaces, in some cases, were sand streaked and, generally, the finish was no better than that obtained by hand puddling methods. However, a substantial saving in cement was obtained because mixed aggregate concrete which would normally be poured with slumps from 20 to 40 in. was vibrated and poured with slumps of 10 to 30 in.

Vibrators used with coarse aggregate concretes have proven entirely satisfactory. It was found that the same machine could be used for all classes of aggregate common in Kansas. Also, that machines with large vibrators puddled coarse aggregate concrete in less time than machines equipped with small size vibrators. On structures of bulk concrete, such as footings, collision walls and thick floors, the vibrator gave excellent results, causing free flow of concrete with slumps which could not be handled satisfactorily without mechanical means of agitation. Care must be used in vibrating rock concrete mixes, to avoid the formation of mortar pockets.

VIBRATORS FOR PAVEMENTS

Several different types of pavement vibrators have been used in the state with both coarse and mixed aggregate materials.

On a mixed aggregate concrete pavement project recently completed, the finishing machine was equipped with a vibrating, oscillating front screed of bull nose design. Three electric motors mounted on this screed set up approximately 4000 vibrations per minute. The front of the 16-in. contact surface of the screed was raised slightly giving a compressive action to the concrete as the

machine proceeded. An oscillating screed was also operated at the rear of the machine.

It was found that mixes with $\frac{3}{8}$ - to $\frac{1}{2}$ -in. slump gave the best results. When wetter mixes were used the concrete slumped away at the crown. Specifications provided for the use of $5\frac{3}{4}$ gal. of water per sack of cement with a maximum cement factor of 1.55 and a minimum cement factor of 1.25. Without the use of a vibrator in this particular case the cement content would have been at or slightly over 1.55 barrels per cubic yard, whereas with the vibrator it was reduced to 1.42 barrels per cubic yard. This is an example of the use of local material, whereas it would have been impossible without a vibrator.

Some special tests and experiments were tried in connection with this project. A cardboard box, the thickness of the slab, was placed on the subgrade, filled with concrete and left in place during the vibrating and finishing of the slab. The box was then very carefully removed and taken to the research laboratory. At the time of removing this box from the slab a flow of mortar through the slab was noticed which indicated that vibration affected the concrete for the full depth of the slab. At the laboratory this block of concrete was cut through the center and the sections thus exposed seemed to show the concrete to be a little denser for about two inches down from the top. However, the remaining depth of the slab looked uniform. Specific gravity of 2.34 was determined on this specimen. The specific gravity of a test beam made on this project was 2.35. It was computed that the theoretical specific gravity should be about 2.40. Cores taken from the slab in five different places showed the concrete to be uniform from top to

bottom with no honeycomb at the bottom. The test beams were made by placing the filled beam molds on the vibrator and allowing them to vibrate for five seconds. The resulting beams were uniform and gave satisfactory four day strengths.

TABLE I

DATA ON TRANSVERSE STRENGTH OF TEST BEAMS
Mixed Aggregate Vibrating Oscillating Front Screed

| Station No | Beam No | Transverse Strength | | | | Yield in Bbls per Cu Yd |
|------------|---------|---------------------|--------------|-------------|--------------|-------------------------|
| | | Age in Days | Lb per Sq In | Age in Days | Lb per Sq In | |
| 76+75 | 1A | 4 | 638 | 10 | 735 | 1 41 |
| 92+40 | 3A | 4 | 625 | 10 | 632 | 1 44 |
| 104+40 | 4A | 4 | 581 | 10 | 629 | 1 44 |
| 118+40 | 5B | 4 | 468 | 10 | 602 | 1 44 |
| 123+50 | 6B | 4 | 534 | 10 | 676 | 1 44 |
| 139+75 | 8A | 4 | 515 | 10 | 690 | 1 44 |
| 149+90 | 9A | 4 | 459 | 10 | 720 | 1 40 |
| 160+80 | 10A | 5 | 555 | 12 | 606 | 1 39 |
| 171+70 | 11A | 5 | 501 | 11 | 584 | 1 39 |
| 184+30 | 12A | 5 | 477 | 10 | 618 | 1 37 |
| 194+60 | 13A | 6 | 613 | 10 | 675 | 1 46 |
| 199+05 | 14A | 6 | 586 | 10 | 671 | 1 39 |
| 209+50 | 15A | 5 | 460 | 11 | 635 | 1 39 |
| 220+80 | 16A | 6 | 663 | 10 | 685 | 1 46 |
| 229+00 | 17A | 3 | 500 | 10 | 642 | 1 40 |
| 240+00 | 18A | 4 | 515 | 10 | 611 | |
| Average | | | | | | 1 417* |

* The average estimated cement factor without vibrator was 1 55 bbl per cu yd
The average specific gravity of cores 2 29

Wire mesh reinforcing placed 2 in from the surface was found to have settled 1 1 in more. Part of this settlement might have been caused by workmen tramping around on the mesh.

While the slump on this project was exceedingly small, we found that the vibrator provided sufficient mortar for all finishing purposes.

TABLE II

DATA ON TRANSVERSE STRENGTH OF TEST BEAMS, COMPRESSION STRENGTHS OF CORES AND BEAM ENDS, CEMENT FACTOR AND SPECIFIC GRAVITY* OF CONCRETE

Coarse Aggregate Finishing Machine with Integral Electric Vibrator

| Station No | Beam No | Transverse Strength | | 90 Day Comp Strength on Beam End in Lb per Sq In | 90 Day Comp Strength on Cores in Lb per Sq In Approx Sta | Yield in Bbls per Cu Yd | |
|------------|---------|---------------------|--------------|--|--|-------------------------|-------|
| | | Age in Days | Lb per Sq In | | | | |
| 471+10 | 1A | 4 | 463 | 10 617 | 4722 | 2765 | 1 52 |
| 461+52 | 2B | 4 | 410 | 10 553 | 4194 | 2651 | 1 52 |
| 445+85 | 4A | 4 | 544 | 10 755 | 6361 | 3677 | 1 33 |
| 429+20 | 5B | | | 10 614 | 5722 | 3204 | 1 34 |
| 411+75 | 7A | 4 | 522 | | 5347 | 3738 | 1 33 |
| 398+52 | 9A | 4 | 452 | 10 556 | 5889 | 3344 | 1 33 |
| 381+88 | 10B | 4 | 440 | 10 518 | 5097 | 5089 | 1 33 |
| 362+75 | 12A | 4 | 595 | 10 651 | 6042 | 4584 | 1 35 |
| 343+35 | 14A | 4 | 438 | 10 584 | 5111 | 5237 | 1 36 |
| 332+50 | 15B | 4 | 501 | 10 589 | 5278 | 3687 | 1 27 |
| 314+50 | 17A | 4 | 586 | 10 638 | 5889 | 3081 | 1 26 |
| 295+00 | 18B | | | 10 600 | 4778 | 3325 | 1 29 |
| 276+05 | 20A | 4 | 485 | 10 517 | 4778 | 2855 | 1 27 |
| 263+60 | 21A | 4 | 473 | 10 637 | 4917 | | |
| 7+90 | 22B | 4 | 549 | 10 592 | 4778 | 3637 | 1 29 |
| 23+85 | 24A | 4 | 561 | 10 561 | 5750 | 3839 | 1 36 |
| 35+00 | 25B | 4 | 562 | 10 583 | 6194 | 5099 | 1 35 |
| 53+60 | 27A | 4 | 571 | 10 649 | 5875 | 4408 | 1 35 |
| 70+20 | 28B | 3 | 560 | 10 577 | 5292 | 4235 | 1 37 |
| 90+00 | 30A | 3 | 517 | 10 497 | 6264 | 4793 | 1 36 |
| 106+60 | 31B | 3 | 508 | 10 584 | 5597 | 3956 | 1 36 |
| 117+50 | 32B | 3 | 475 | 10 479 | 4444 | 5048 | 1 36 |
| 136+40 | 34A | 3 | 538 | 10 700 | 5639 | 5929 | 1 36 |
| 154+70 | 35B | 4 | 551 | 10 670 | 6014 | 5079 | 1 36 |
| 168+50 | 37A | 3 | 578 | 10 678 | 5944 | 3723 | 1 37 |
| 187+75 | 38B | 3 | 472 | 10 564 | 6472 | 3906 | 1 36 |
| 207+35 | 40A | 3 | 514 | 11 597 | 5639 | 3622 | 1 34 |
| 226+35 | 41B | 4 | 569 | 11 670 | 5903 | 4073 | 1 35 |
| 244+50 | 43A | 3 | 485 | 10 534 | 5861 | 4024 | 1 36 |
| 259+90 | 44B | 3 | 505 | 10 595 | 5921 | 5213 | 1 36 |
| Average | | | | | 5524 | 4063 | 1 35† |

* Average specific gravities were beams 2 31, cores 2 28

† The estimated average cement factor without vibrator was 1 43 bbl per cu yd

It was stated before that trouble was experienced from flattening of the crown. This might be due to the fact that the vibrator manufacturers were not particular enough in placing the correct curvature in the vibrator screeds. Table I gives transverse strengths found on test beams. Sufficient time has not elapsed to make available compression tests on beam ends or cores.

On another project an electric finishing machine with integral electric vibrator was used. In addition to the vibrating screed the equipment carried a front and rear oscillating screed. This machine weighed approximately 6000 pounds. The vibrator was run with a frequency of between 3500 and 3700 V P M. This vibrator furnished sufficient mortar to finish satisfactorily coarse aggregate concrete with a slump of $1\frac{1}{2}$ in. The slump was reduced approximately one inch over that which would have been necessary had the vibrator not been used. The cement content on this project averaged 1.35 barrels per cubic yard of concrete. Assuming that no vibrator had been used approximately 1.45 barrels of cement per cubic yard of concrete would have been required. On this type of machine the manufacturer likewise had paid little attention to the crown in the vibrator screed. For this reason vibration tended to eliminate the crown at the quarter points and gave some difficulty in obtaining a satisfactory curvature for the surface of the pavement. Due to the weight of the finishing machine, coupled with a sandy soil condition, the forms were difficult to hold and some settlement resulted. Inspection of cores taken from this particular pavement revealed very good results had been obtained. Cores tested for compression also gave average strengths. Table II gives transverse

strength of test beams, compression strengths of core and beam ends and specific gravity of concrete.

This type machine was also used on another coarse aggregate project.

TABLE III

DATA ON TRANSVERSE STRENGTH OF BEAMS, COMPRESSION STRENGTH OF BEAM ENDS AND CORES, CEMENT FACTOR AND SPECIFIC GRAVITY*

Coarse Aggregate Finishing Machine with Integral Electric Vibrator

| Station No | Beam No | Transverse Strength | | 90 Day Comp Strength on Beam End in Lb per Sq In | 90 Day Comp Strength on Cores in Lb per Sq In Approx Sta | Yield in Bbls per Cu Yd | |
|------------|---------|---------------------|--------------|--|--|-------------------------|--------|
| | | Age in Days | Lb per Sq In | | | | |
| 482+90 | 1A | 5 | 498 | 10605 | 5111 | 4564 | 1 33 |
| 488+50 | 2A | 4 | 598 | 10694 | 4778 | 4395 | 1 33 |
| 495+00 | 3A | 4 | 691 | 10840 | 4889 | 4648 | 1 275 |
| 508+00 | 4B | 4 | 637 | 10670 | 5694 | 3913 | 1 26 |
| 515+35 | 5B | 4 | 641 | 10740 | 4750 | 3672 | 1 26 |
| 520+00 | 7A | 3 | 592 | 6717 | 7069 | 5246 | 1 26 |
| 526+20 | 8A | 6 | 796 | | 7097 | 4151 | 1 26 |
| 532+00 | 10A | 4 | 696 | 7663 | 6486 | 4432 | 1 28 |
| 541+50 | 12A | 4 | 702 | 7774 | 6014 | 4360 | |
| 548+00 | 13B | 4 | 642 | 7786 | 6486 | 4538 | 1 26 |
| 558+50 | 14B | 4 | 568 | 7696 | 4750 | 3654 | 1 26 |
| 568+75 | 15B | 4 | 562 | 7620 | 5194 | 4233 | 1 26 |
| 579+35 | 16B | 4 | 629 | 7630 | 5833 | 4961 | 1 26 |
| 585+00 | 17B | 5 | 626 | 7657 | 5292 | 4651 | 1 26 |
| 589+00 | 19A | 3 | 649 | 8737 | 6417 | 3612 | 1 26 |
| Average | | | | | 5724 | 4335 | 1 272† |

* Average specific gravities were beams 2.37, cores 2.28.

† The average estimated cement factor without vibration was 1.38 bbl per cu yd.

The average mix was 1:1.98:4.56, proportioned by weight, with a cement content of 1.26 and 1.27 barrels per cubic yard. With this mix, usually considered harsh, no scarcity of mortar was noticeable and the finish was satisfactory. Also good results were obtained from test

beams broken at four days. The beams seemed to indicate somewhat higher earlier strength than that obtained with ordinary finishing equipment. There was about $\frac{1}{8}$ in settlement in the forms due to the weight of the machine. Table III

TABLE IV

DATA ON TRANSVERSE STRENGTH OF BEAMS
COMPRESSION STRENGTH OF BEAM ENDS
AND CORES, CEMENT FACTOR AND SPECIFIC
GRAVITY*

Coarse Aggregate Vibrator between Front
and Rear Oscillating Screeds, Gasoline
Engine

| Station No | Beam No | Transverse Strength | | 90 Day Comp Strength on Beam End in Lb per Sq. In | 90 Day Comp Strength on Cores in Lb per Sq. In Approx Sta | Yield in Bbls per Cu Yd | | | |
|------------|---------|------------------------|---------------|---|---|----------------------------|------|---|------|
| | | Age in Days | Lb per Sq. In | | | | | | |
| 177+25 | 1-B-1 | 5 | 429 | 10 | 596 | 5528 | 2372 | 1 | 38 |
| 165+50 | 3A | 6 | 547 | 4 | | 4833 | 2938 | 1 | 39 |
| 151+70 | 4B | 6 | 376 | 2 | | 6139 | 3573 | 1 | 44 |
| 134+06 | 6A | 10 | 529 | | | 4722 | 3462 | 1 | 43 |
| 125+72 | 7B | 10 | 534 | | | 5639 | 3447 | 1 | 43 |
| 116+10 | 8B | 10 | 521 | | | 5333 | | 1 | 43 |
| 104+46 | 9B | 6 | 422 | 10 | 535 | 5667 | 3203 | 1 | 405 |
| 98+40 | 10B | 10 | 546 | | | 4445 | 2398 | 1 | 43 |
| 80+25 | 12A | 6 | 458 | | | 4806 | | 1 | 45 |
| 69+64 | 13A | 6 | 560 | 10 | 510 | 5139 | | 1 | 48 |
| 60+82 | 14A | 7 | 413 | | | 6445 | | 1 | 43 |
| 50+97 | 15A | 6 | 498 | 10 | 573 | 5834 | | 1 | 41 |
| 47+20 | 16A | 6 | 345 | | | 4028 | | 1 | 51 |
| Average | | | | | | 5274 | 3056 | 1 | 431† |

* Average specific gravity of cores was 2.15

† The average estimated cement factor without vibration was 1.53 bbl per cu yd

shows strengths obtained on test specimens

On a three mile section of mixed aggregate pavement a finishing machine was used with vibrator attachment. This vibrator was operated by a separate 15 horsepower four cylinder gasoline engine

with a frequency of between 3600 and 4200 V P M. The vibrator was mounted between regular front and rear oscillating screeds. The surface of the vibrator in contact with the concrete was concave with well rounded ends. It had a contact surface of about 6 in. Slumps as low as $\frac{1}{8}$ in. were tried but discarded because apparently there was not enough moisture present to lubricate the materials. The first cores taken with this extremely small slump were found to be porous. In this machine as in others, the manufacturer had failed to recognize the importance of building the crown of the vibrator beam the same as the specified crown of the finished pavement. It might be desirable, in some materials, to have a crown in excess of that desired by the finished product. Another criticism of this particular vibrator was that concrete would pile on top, thus dampening the vibrating effect. It was necessary at all times to keep all excess concrete from the top of the vibrator beam.

Cores obtained from the slab and tested at the research laboratory, appeared to have abnormal porosity and low density. The specimens seemed to show denser concrete near the surface and the bottom of the cores, than was found about the center. Due to the fact that this machine did not produce satisfactory concrete, blanket approval of its use on mixed aggregate pavements has been withheld until further experiments can be made with it.

Later this same finisher and vibrator was used on a project where coarse aggregate was used. The machine was operated at a frequency of about 3500 V P M. The proportions of materials were finally adjusted to a mix of 1.2 1.4 35, proportioned by weight, with a 1.29 cement factor. The slump used was from $1\frac{1}{2}$ to

TABLE V

DATA ON TRANSVERSE STRENGTH OF BEAMS, COMPRESSION STRENGTH OF BEAM ENDS AND CORES, SPECIFIC GRAVITY* AND CEMENT FACTOR

Coarse Aggregate Vibrator between Front and Rear Oscillating Screeds, Gasoline Engine

| Station No | Beam No | Transverse Strength | | | | 90 Day Comp Strength on Beam End in Lb per Sq In | 90 Day Comp Strength on Cores in Lb per Sq In Approx Sta | Yield in Bbls per Cu Yd |
|------------|---------|---------------------|--------------|-------------|--------------|--|--|-------------------------|
| | | Age in Days | Lb per Sq In | Age in Days | Lb per Sq In | | | |

Section A

| | | | | | | | | | |
|--------|-----|---|-----|----|-----|------|------|---|----|
| 649+00 | 1B | 5 | 650 | 10 | 644 | 6819 | | 1 | 32 |
| 642+25 | 3B | 5 | 561 | 10 | 628 | 5333 | 4185 | 1 | 32 |
| 631+40 | 5B | 6 | 511 | 10 | 625 | 4500 | | 1 | 34 |
| 620+50 | 7A | 4 | 525 | 10 | 799 | 6167 | 4350 | 1 | 34 |
| 609+40 | 8B | 4 | 484 | 10 | 625 | | 4396 | 1 | 34 |
| 600+50 | 9B | 4 | 572 | 10 | 576 | 5611 | 3320 | 1 | 30 |
| 588+50 | 10B | 4 | 556 | 10 | 563 | 4519 | 3232 | 1 | 30 |
| 575+00 | 12A | 4 | 599 | 10 | 701 | 4944 | 3378 | 1 | 27 |
| 557+80 | 14A | 4 | 571 | 10 | 705 | 4653 | 4604 | 1 | 28 |
| 549+00 | 15B | 4 | 623 | 11 | 749 | 5639 | 2963 | 1 | 28 |
| 542+50 | 16B | 4 | 374 | 10 | 707 | 5306 | 4291 | 1 | 27 |
| 530+00 | 17B | 4 | 396 | | | 4111 | 3848 | 1 | 30 |
| 520+00 | 18B | 4 | 432 | | | 3139 | 3372 | 1 | 30 |
| 511+00 | 19B | 7 | 511 | | | 3222 | | 1 | 29 |
| 502+00 | 20B | 6 | 595 | | | 4972 | 3175 | 1 | 29 |
| 493+00 | 22B | 4 | 533 | 10 | 574 | 5056 | 3302 | 1 | 32 |

Section B

| | | | | | | | | | |
|--------|-----|---|-----|----|-----|------|------|---|----|
| 474+00 | 1B | 4 | 547 | 10 | 654 | 6528 | | 1 | 31 |
| 263+00 | 2A | 4 | 639 | 10 | 751 | 5875 | 3494 | 1 | 31 |
| 278+00 | 4A | 4 | 608 | 10 | 683 | 6486 | 4395 | 1 | 29 |
| 291+00 | 5B | 4 | 652 | 10 | 537 | 5930 | 2989 | 1 | 27 |
| 300+00 | 6B | 4 | 586 | 11 | 606 | 5250 | 3440 | 1 | 27 |
| 311+00 | 7B | 4 | 623 | 10 | 630 | 6681 | 4161 | 1 | 27 |
| 321+00 | 8B | 4 | 512 | 10 | 478 | 4778 | 4626 | 1 | 27 |
| 330+00 | 9B | 4 | 641 | 10 | 554 | 5819 | 3327 | 1 | 28 |
| 340+00 | 10B | 4 | 608 | 10 | 728 | 6444 | 4191 | 1 | 28 |
| 349+00 | 11B | 5 | 672 | 10 | 551 | 7153 | 4987 | 1 | 28 |
| 359+00 | 12B | 5 | 533 | 9 | 688 | 6278 | 4894 | 1 | 28 |
| 370+00 | 13B | 4 | 575 | 10 | 662 | 4861 | | 1 | 28 |
| 382+00 | 14B | 4 | 575 | 10 | 642 | 6236 | 4086 | 1 | 28 |

* Average specific gravities were beams 2 35, cores 2 31

TABLE V—Concluded

| Station No | Beam No | Transverse Strength | | | | 90 Day Comp Strength on Beam End in Lb per Sq In | 90 Day Comp Strength on Cores in Lb per Sq In Approx Sta | Yield in Bbls per Cu Yd |
|------------|---------|---------------------|--------------|-------------|--------------|--|--|-------------------------|
| | | Age in Days | Lb per Sq In | Age in Days | Lb per Sq In | | | |

Section B—Concluded

| | | | | | | | | | |
|---------|-----|---|-----|----|-----|------|------|---|------|
| 391+00 | 15B | 4 | 597 | 10 | 652 | 6083 | 4070 | 1 | 28 |
| 404+00 | 16B | 4 | 560 | 10 | 504 | 6264 | 2500 | 1 | 28 |
| 410+00 | 17A | 4 | 606 | 10 | 693 | 6083 | 3542 | 1 | 28 |
| 420+00 | 18B | 4 | 575 | 10 | 711 | 5139 | | 1 | 26 |
| 430+00 | 19B | 4 | 540 | 10 | 693 | 4944 | 3726 | 1 | 26 |
| 440+00 | 20B | 4 | 541 | 10 | 648 | 5000 | 4123 | 1 | 26 |
| 448+50 | 21B | 4 | 659 | 10 | 754 | 5472 | | 1 | 26 |
| 460+00 | 23A | 5 | 627 | 10 | 670 | 5083 | 3627 | 1 | 26 |
| Average | | | | | | 5455 | 3820 | 1 | 288† |

† Average estimated cement factor without vibration was 1 42 bbl per cu yd

TABLE VI

DATA ON TRANSVERSE STRENGTH OF BEAMS USING DIFFERENT WATER CEMENT RATIOS
Coarse Aggregate Vibrator between Front and Rear Oscillating Screeds, Gasoline Engine

| Station No | Beam No | Transverse Strength | | | | Yield in Bbls per Cu Yd |
|------------|---------|---------------------|--------------|-------------|--------------|-------------------------|
| | | Age in Days | Lb per Sq In | Age in Days | Lb per Sq In | |
| 75+98* | 18B | 5 | 545 | 10 | 612 | 1 16 |
| 274+08* | 19B | 5 | 485 | 10 | 582 | 1 15 |
| Average | | | | | | 1 155 |
| 57+30† | 22A | 5 | 562 | 10 | 529 | 1 35 |
| 255+00† | 22B | 5 | 578 | 10 | 700 | 1 35 |
| 252+90† | 23A | 5 | 632 | 10 | 552 | 1 35 |
| 249+90† | 24A | 5 | 455 | 10 | 671 | 1 34 |
| 248+50† | 24B | 5 | 513 | 10 | 619 | 1 35 |
| Average | | | | | | 1 35† |

* 7 00 gals water used

† 5 75 gals water used

‡ The average estimated cement factor without vibrator was 1 40 bbl per cu yd

2 in This gave enough mortar for all finishing purposes Attempts to lower the moisture content and cement factor resulted in a mix that was too dry and harsh to finish properly Comparison by computation was made to determine savings due to the vibrator The vibrator allowed the use of more stone, required less sand and from 3 to 6 per cent less cement All beams tested showed required strength or better Check levels taken on the forms indicated that the vibrator caused the forms to settle about $\frac{1}{8}$ in In most instances where a vibrator is used in connection with the finishing machine, settlement of the forms has been found and it is a factor that will have to be taken into consideration when preparing the subgrade Tables IV and V show various strengths obtained with test specimens

This same finisher was again used in vibrating some sections on a five mile experimental concrete pavement project This job was particularly interesting for the reason that a Goldbeck pressure cell was installed in the subgrade in an effort to find if the vibrations were carried through the concrete The machine was stopped so that the vibrator was directly over the instrument which was adjusted so the light was not illuminated When the vibrator was started the light immediately came on As the vibrations ceased the light flickered, showing that vibrations were actually being carried through to the subgrade The mix used in this particu-

lar case was rock with $33\frac{1}{2}$ per cent sand by volume with 7 gal of water per sack of cement and the slump was 2 in This test was repeated again with $5\frac{3}{4}$ gal of water with a 1-in slump and 30 per cent sand by volume The light was distinctly weaker than during the first experiment, indicating that vibrations were not being carried through to the base with as much intensity as was noted with the wetter mix The frequency on this machine was between 3600 and 4200 V P M

In this same experiment a tamper was used As the tamper approached the cell the light came on giving a more intense light than when used with the vibrator Table VI gives various strengths obtained on test specimens

CONCLUSIONS

From these observations the writer believes that the following conclusions are justified

1 That vibrators used in connection with structural or pavement concretes will allow a reduction in cement content or if the aggregates are kept constant will give greater strength due to decrease in the amount of water needed

2 The amount of vibration must be governed according to the particular aggregates used

3 That the use of vibrators will in no way permit any laxness in the writing of material specifications

4 With pavement vibrators form settlement must be taken into consideration